GEOTECHNICAL INVESTIGATION
PROPOSED MORENO VALLEY LOGISTICS CENTER

SWC Krameria Avenue and Indian Street
Moreno Valley, California
for
ProLogis

SoCalGeo
SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

March 24, 2015

SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

ProLogis
2817 East Cedar Street, Suite 200
Ontario, California 91761

Attention: Mr. Scott Mulkay
Vice President – Regional Development Manager

Project No.: **14G160-1R**

Subject: **Geotechnical Investigation**
Proposed Moreno Valley Logistics Center
SWC Krameria Avenue and Indian Street
Moreno Valley, California

Gentlemen:

In accordance with your request, we have conducted a geotechnical investigation at the subject site. We are pleased to present this report summarizing the conclusions and recommendations developed from our investigation.

We sincerely appreciate the opportunity to be of service on this project. We look forward to providing additional consulting services during the course of the project. If we may be of further assistance in any manner, please contact our office.

Respectfully Submitted,

SOUTHERN CALIFORNIA GEOTECHNICAL, INC.

Robert G. Trazo, GE 2655
Principal Engineer

REGISTERED PROFESSIONAL ENGINEER
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1.0 EXECUTIVE SUMMARY

Presented below is a brief summary of the conclusions and recommendations of this investigation. Since this summary is not all inclusive, it should be read in complete context with the entire report.

Site Preparation

- Initial site preparation should include stripping of any surficial vegetation from the site. At the time of our investigation, ground surface cover consists of moderate amounts of native grass and weed growth. This material should be removed and disposed of off-site.
- The subject site is generally underlain by native alluvial soils possessing high collapse potential and moderate susceptibility to consolidation extending to depths of 3 to 4± feet as well as lower strength alluvial soils extending to depths of 3 to 4± feet. The underlying native soils generally possess higher strengths and generally more favorable consolidation and collapse characteristics.
- Remedial grading is recommended to be performed within the new building pad areas. The existing soils within the building areas should be overexcavated to a depth of 3 feet below existing grade and to a depth of 3 feet below proposed pad grades. The soils within the proposed foundation influence zones should be overexcavated to a depth of 2 feet below proposed foundation bearing grades.
- After the recommended overexcavation has been completed, the resulting subgrade soils should be evaluated by the geotechnical engineer to identify any additional soils that should be overexcavated. The resulting subgrade should then be scarified to a depth of 10 to 12 inches and thoroughly moisture conditioned to 2 to 4 percent above optimum moisture content. The resulting subgrade should then be recompacted to at least 90 percent of the ASTM D-1557 maximum dry density. The previously excavated soils may then be replaced as compacted structural fill.
- The new parking area subgrade soils are recommended to be scarified to a depth of 12± inches, moisture conditioned to 2 to 4 percent above optimum, and recompacted to at least 90 percent of the ASTM D-1557 maximum dry density.

Building Foundations

- Conventional shallow foundations, supported in newly placed compacted fill.
- 2,500 lbs/ft^2 maximum allowable soil bearing pressure.
- Reinforcement consisting of at least four (4) No. 5 rebars (2 top and 2 bottom) in strip footings due to the presence of low to medium expansive soils. Additional reinforcement may be necessary for structural considerations.

Building Floor Slabs

- Conventional Slab-on-Grade, 5 inches thick.
- Minimum slab reinforcement: No. 3 bars at 18-inches on center, in both directions due to low to medium expansive soils. The actual floor slab reinforcement should be determined by the structural engineer, based upon the imposed loading.

Pavements

ASPHALT PAVEMENTS (R = 35)					
Materials	**Thickness (inches)**				
	Auto Parking and Auto Drive Lanes (TI = 4.0 to 5.0)	Truck Traffic			
		TI = 6.0	TI = 7.0	TI = 8.0	TI = 9.0
Asphalt Concrete	3	3½	4	5	5½
Aggregate Base	5	7	9	10	11
Compacted Subgrade	12	12	12	12	12

PORTLAND CEMENT CONCRETE PAVEMENTS (R = 35)				
Materials	**Thickness (inches)**			
	Autos and Light Truck Traffic (TI = 6.0)	Truck Traffic		
		TI = 7.0	TI = 8.0	TI = 9.0
PCC	5	6½	8	9
Compacted Subgrade (95% minimum compaction)	12	12	12	12

2.0 SCOPE OF SERVICES

The scope of services performed for this project was in accordance with our Proposal No. 14P266, dated June 4, 2014. The scope of services included a visual site reconnaissance, subsurface exploration, field and laboratory testing, and geotechnical engineering analysis to provide criteria for preparing the design of the building foundations, building floor slabs, and parking lot pavements along with site preparation recommendations and construction considerations for the proposed development. The evaluation of the environmental aspects of this site was beyond the scope of services for this geotechnical investigation.

3.0 SITE AND PROJECT DESCRIPTION

3.1 Site Conditions

The subject site is located on the west side of Indian Street southwest of the intersection of Indian Street and Krameria Avenue in Moreno Valley, California. The site is bounded to the north by a commercial/industrial building and vacant land, to the east by Indian Street, to the west by vacant land, commercial/industrial buildings, and Heacock Street. The Perris Valley Storm Drain channel transects the site in a northwest to southeast direction. The general location of the site is illustrated on the Site Location Map included as Plate 1 in Appendix A of this report.

The overall site in an "L"-shaped property, 82.7± acres in size. The overall site is comprised of four (4) separate parcels which are divided by the northwest-to-southeast trending Perris Valley Storm Drain channel. Parcels 1 and 2 are located on the east side of the channel and are 58.7± and 6.9± acres in size, respectively. Parcel 3 is located to the southwest of the storm drain channel and is approximately 6.4 acres in size. Parcel 4 is located to the northwest of the channel and is 9.4± acres in size. Ground surface cover throughout the overall site consists of exposed soil with sparse to moderate native grass and weed growth. Several regions of the property appear to have been previously disked.

Detailed topographic information was not available at the time of this report. However, based on visual observations made at the time of our investigation, the site topography within Parcels 1 and 2 generally slopes downward to the southwest at an estimated gradient of 2 to 3± percent, with an estimated elevation differential of 25± feet across the Parcel 1 site. The site topography within Parcels 3 and 4 generally slopes downward to the south at an estimated gradient of less than 1 percent. Topographic relief across Parcels 3 and 4 are on the order of 10± feet and 4± feet, respectively.

3.2 Proposed Development

Based on the conceptual site plan provided to our office by the client, the overall site will be developed with four (4) commercial/industrial buildings, identified and Buildings 1 through 4. The buildings will generally encompass the central regions of their respective parcels. The buildings will range from 93,665± ft^2 to 1,333,586± ft^2 in size and will include of 5,000± ft^2 to 10,000± ft^2 of office space. Building 1 will be constructed in a cross-dock configuration, with loading docks on the east and west sides of the building. Buildings 2 and 4 will be constructed with loading docks along the west and east building lines, respectively. Loadings will also be constructed on the north and east sides of Building 3. The areas surrounding the buildings are expected to be developed with asphaltic concrete pavements in the automobile parking and drive lanes and Portland cement concrete (PCC) in the loading dock areas. It is assumed that landscape planters and areas of concrete flatwork with will be located in limited areas throughout the site. The proposed development will also include the construction of new public streets, located at the north and east property lines.

Detailed structural information has not been provided. It is assumed that the new buildings will be single-story structures of tilt-up concrete construction, supported on conventional shallow foundations with concrete slab-on-grade floors. The construction may include second floor mezzanine offices. Based on the assumed construction, maximum column and wall loads are expected to be on the order of 80 kips and 3 to 5 kips per linear foot, respectively.

Based on the existing topography, cuts and fills of 3 to 6± feet are expected to be necessary to achieve the new site grades within the building areas. No significant below grade construction such as basements or crawl spaces is anticipated.

4.0 SUBSURFACE EXPLORATION

4.1 Scope of Exploration/Sampling Methods

The subsurface exploration conducted for this project consisted of twenty-two (22) borings advanced to depths of 5 to 30± feet below existing site grades. All of the borings were logged during drilling by a member of our staff.

The borings were advanced with hollow-stem augers, by a truck-mounted drilling rig. Representative bulk and in-situ soil samples were taken during drilling. Relatively undisturbed in-situ samples were taken with a split barrel "California Sampler" containing a series of one inch long, 2.416± inch diameter brass rings. This sampling method is described in ASTM Test Method D-3550. In-situ samples were also taken using a 1.4± inch inside diameter split spoon sampler, in general accordance with ASTM D-1586. Both of these samplers are driven into the ground with successive blows of a 140-pound weight falling 30 inches. The blow counts obtained during driving are recorded for further analysis. Bulk samples were collected in plastic bags to retain their original moisture content. The relatively undisturbed ring samples were placed in molded plastic sleeves that were then sealed and transported to our laboratory.

The approximate locations of the borings are indicated on the Boring Location Plan, included as Plate 2 in Appendix A of this report. The Boring Logs, which illustrate the conditions encountered at the boring locations, as well as the results of some of the laboratory testing, are included in Appendix B.

4.2 Geotechnical Conditions

Alluvium

Native alluvial soils were encountered at the ground surface of all of the exploratory borings. The alluvial soils generally consist of very stiff to hard sandy clays, clayey silts and silty clays as well as medium dense to very dense sands, silty sands and clayey sands extending to the maximum depth explored of 30± feet. The results of the laboratory testing indicate that a significant amount of the near surface soils were in a dry to damp condition.

Groundwater

At Boring No. B-19, free water was encountered at a depth of 27± feet below existing site grades. However, the remaining twenty-one (21) borings did not encounter any groundwater. It is expected that the groundwater encountered at Boring No. B-19 was localized perched water, sitting above an impermeable soil layer. Based on the lack of any water within the remaining borings, and the moisture contents of the recovered soil samples, the static groundwater table is considered to have existed at a depth in excess of 30± feet at the time of the subsurface exploration.

5.0 LABORATORY TESTING

The soil samples recovered from the subsurface exploration were returned to our laboratory for further testing to determine selected physical and engineering properties of the soils. The tests are briefly discussed below. It should be noted that the test results are specific to the actual samples tested, and variations could be expected at other locations and depths.

Classification

All recovered soil samples were classified using the Unified Soil Classification System (USCS), in accordance with ASTM D-2488. Field identifications were then supplemented with additional visual classifications and/or by laboratory testing. The USCS classifications are shown on the Boring Logs and are periodically referenced throughout this report.

In-situ Density and Moisture Content

The density has been determined for selected relatively undisturbed ring samples. These densities were determined in general accordance with the method presented in ASTM D-2937. The results are recorded as dry unit weight in pounds per cubic foot. The moisture contents are determined in accordance with ASTM D-2216, and are expressed as a percentage of the dry weight. These test results are presented on the Boring Logs.

Soluble Sulfates

Representative samples of the near-surface soils were submitted to a subcontracted analytical laboratory for determination of soluble sulfate content. Soluble sulfates are naturally present in soils, and if the concentration is high enough, can result in degradation of concrete which comes into contact with these soils. The results of the soluble sulfate testing are presented below, and are discussed further in a subsequent section of this report.

Sample Identification	Soluble Sulfates (%)	Sulfate Classification
B-1 @ 0 to 5 feet	0.028	Negligible
B-10 @ 0 to 5 feet	0.022	Negligible
B-13 @ 0 to 5 feet	0.013	Negligible

Consolidation

Selected soil samples have been tested to determine their consolidation potential, in accordance with ASTM D-2435. The testing apparatus is designed to accept either natural or remolded samples in a one-inch high ring, approximately 2.416 inches in diameter. Each sample is then loaded incrementally in a geometric progression and the resulting deflection is recorded at selected time intervals. Porous stones are in contact with the top and bottom of the sample to permit the addition or release of pore water. The samples are typically inundated with water at an intermediate load to determine their potential for collapse or heave. The results of the consolidation testing are plotted on Plates C-1 through C-20 in Appendix C of this report.

Maximum Dry Density and Optimum Moisture Content

Three (3) representative bulk samples were tested for their maximum dry densities and optimum moisture contents. The results were obtained using the Modified Proctor procedure, per ASTM D-1557. These test results are enclosed in presented on Plates C-21 through C-23 in Appendix C of this report. This test is generally used to compare the in-situ densities of undisturbed field samples, and for later compaction testing. Additional testing of other soil types or soil mixes may be necessary at a later date.

Expansion Index

The expansion potential of the on-site soils was determined in general accordance with ASTM D-4829. The testing apparatus is designed to accept a 4-inch diameter, 1-in high, remolded sample. The sample is initially remolded to 50± 1 percent saturation and then loaded with a surcharge equivalent to 144 pounds per square foot. The sample is then inundated with water, and allowed to swell against the surcharge. The resultant swell or consolidation is recorded after a 24-hour period. The results of the EI testing are as follows:

Sample Identification	**Expansion Index**	**Expansion Potential**
B-4 @ 0 to 5 feet	66	Medium
B-10 @ 0 to 5 feet	33	Low
B-13 @ 0 to 5 feet	0	Very Low
B-16 @ 0 to 5 feet	1	Very Low

Resistivity and pH Testing

The resistivity of the soils is a measure of their potential to attack buried metal improvements such as utility lines. The results of the resistivity and pH testing are presented below. These test results are as follows:

Sample Identification	**Resistivity (ohm-cm)**	**pH**
B-3 @ 0 to 5 feet	1,270	7.6

6.0 CONCLUSIONS AND RECOMMENDATIONS

Based on the results of our review, field exploration, laboratory testing and geotechnical analysis, the proposed development is considered feasible from a geotechnical standpoint. The recommendations contained in this report should be taken into the design, construction, and grading considerations. The recommendations are contingent upon all grading and foundation construction activities being monitored by the geotechnical engineer of record. The Grading Guide Specifications, included as Appendix D, should be considered part of this report, and should be incorporated into the project specifications. The contractor and/or owner of the development should bring to the attention of the geotechnical engineer any conditions that differ from those stated in this report, or which may be detrimental for the development.

6.1 Seismic Design Considerations

The subject site is located in an area which is subject to strong ground motions due to earthquakes. The performance of a site specific seismic hazards analysis was beyond the scope of this investigation. However, numerous faults capable of producing significant ground motions are located near the subject site. Due to economic considerations, it is not generally considered reasonable to design a structure that is not susceptible to earthquake damage. Therefore, significant damage to structures may be unavoidable during large earthquakes. The proposed structures should, however, be designed to resist structural collapse and thereby provide reasonable protection from serious injury, catastrophic property damage and loss of life.

Faulting and Seismicity

Research of available maps indicates that the subject site is not located within an Alquist-Priolo Earthquake Fault Zone. Therefore, the possibility of significant fault rupture on the site is considered to be low.

Seismic Design Parameters

The 2013 California Building Code (CBC) was adopted by all municipalities within Southern California on January 1, 2014. The CBC provides procedures for earthquake resistant structural design that include considerations for on-site soil conditions, occupancy, and the configuration of the structure including the structural system and height. The seismic design parameters presented below are based on the soil profile and the proximity of known faults with respect to the subject site.

The 2013 CBC Seismic Design Parameters have been generated using U.S. Seismic Design Maps, a web-based software application developed by the United States Geological Survey. This software application, available at the USGS web site, calculates seismic design parameters in accordance with the 2013 CBC, utilizing a database of deterministic site accelerations at 0.01 degree intervals. The table below is a compilation of the data provided by the USGS application. A copy of the output generated from this program is included in Appendix E of this report. A copy of the Design Response Spectrum, as generated by the USGS application is also included in Appendix E. Based on this output, the following parameters may be utilized for the subject site:

2013 CBC SEISMIC DESIGN PARAMETERS

Parameter		Value
Mapped Spectral Acceleration at 0.2 sec Period	S_S	1.500
Mapped Spectral Acceleration at 1.0 sec Period	S_1	0.600
Site Class	---	D
Site Modified Spectral Acceleration at 0.2 sec Period	S_{MS}	1.500
Site Modified Spectral Acceleration at 1.0 sec Period	S_{M1}	0.900
Design Spectral Acceleration at 0.2 sec Period	S_{DS}	1.000
Design Spectral Acceleration at 1.0 sec Period	S_{D1}	0.600

Liquefaction

Liquefaction is the loss of the strength in generally cohesionless, saturated soils when the pore-water pressure induced in the soil by a seismic event becomes equal to or exceeds the overburden pressure. The primary factors which influence the potential for liquefaction include groundwater table elevation, soil type and grain size characteristics, relative density of the soil, initial confining pressure, and intensity and duration of ground shaking. The depth within which the occurrence of liquefaction may impact surface improvements is generally identified as the upper 50 feet below the existing ground surface. Liquefaction potential is greater in saturated, loose, poorly graded fine sands with a mean (d_{50}) grain size in the range of 0.075 to 0.2 mm (Seed and Idriss, 1971). Clayey (cohesive) soils or soils which possess clay particles (d<0.005mm) in excess of 20 percent (Seed and Idriss, 1982) are generally not considered to be susceptible to liquefaction, nor are those soils which are above the historic static groundwater table.

The Riverside County Land Information System indicates that the subject site is located within a zone of low liquefaction susceptibility. In addition, the subsurface conditions encountered at the boring locations are not considered to be susceptible to liquefaction. These conditions consist of medium dense well-graded granular soils, and the lack of a shallow groundwater table. Based on these conditions, liquefaction is not considered to be a design concern for this project.

6.2 Geotechnical Design Considerations

General

Native alluvial soils were encountered at the ground surface at all of the borings. The native alluvial soils at depths of 2 to 4 feet had generally lower strengths than the native alluvial soils at greater depths. The native alluvial soils at depths of 2 to 4 feet also possessed significant collapse potential and possessed moderate susceptibility to load-induced consolidation. In addition, numerous samples of the near surface clayey soils were dry. Dry, clayey, near-surface soils possess a moderate potential for swelling and soil heave when exposed to cyclical wetting and drying. Based on these conditions, remedial grading is considered warranted within the proposed building areas in order to remove and replace the surficial soils as compacted structural fill. The proposed remedial grading will also result in placement of a uniform blanket of structural fill below the foundations and floor slabs of the new structures.

Settlement

The recommended remedial grading will remove a portion of the near surface alluvial soils, and replace these materials as compacted structural fill. The native soils that will remain in place below the depth of recommended overexcavation possess favorable consolidation/collapse characteristics. Therefore, following completion of the recommended grading, post-construction settlements are expected to be within tolerable limits.

Expansion

The near surface soils at this site generally consist of sandy clays, silty clays, and clayey sands. Laboratory testing indicates that these materials have a very low to medium expansion potential (EI = 0 to 66). Based on the presence of expansive soils, special care should be taken to properly moisture condition and maintain adequate moisture content within all subgrade soils as well as newly placed fill soils. The foundation and floor slab design recommendations contained within this report are made in consideration of the expansion index test results. It is recommended that additional expansion index testing be conducted at the completion of rough grading to verify the expansion potential of the as-graded building pads.

Soluble Sulfates

The results of the soluble sulfate testing indicate that the selected sample of the on-site soils contains a negligible concentration of soluble sulfates, in accordance with American Concrete Institute (ACI) guidelines. Therefore, specialized concrete mix designs are not considered to be necessary, with regard to sulfate protection purposes. It is, however, recommended that additional soluble sulfate testing be conducted at the completion of rough grading to verify the soluble sulfate concentrations of the soils which are present at pad grade within the building areas.

Shrinkage/Subsidence

Based on the results of the laboratory testing, removal and recompaction of the near surface native soils is estimated to result in an average shrinkage of 12 to 16 percent. Removal and recompaction of the existing fill soils is expected to result in an average shrinkage of 5 to 10± percent. Minor ground subsidence is expected to occur in the soils below the zone of removal, due to settlement and machinery working. The subsidence is estimated to be 0.1 ±feet. This estimate may be used for grading in areas that are underlain by native alluvial soils. These estimates are based on previous experience and the subsurface conditions encountered at the boring locations. The actual amount of subsidence is expected to be variable and will be dependent on the type of machinery used, repetitions of use, and dynamic effects, all of which are difficult to assess precisely.

Grading and Foundation Plan Review

No grading and foundation plans were available at the time of this report. It is therefore recommended that we be provided with copies of the preliminary plans, when they become available, for review with regard to the conclusions, recommendations, and assumptions contained within this report.

6.3 Site Grading Recommendations

The grading recommendations presented below are based on the subsurface conditions encountered at the boring locations and our understanding of the proposed development. We recommend that all grading activities be completed in accordance with the Grading Guide Specifications included as Appendix D of this report, unless superseded by site-specific recommendations presented below.

Site Stripping and Demolition

Initial site stripping should include removal of any surficial vegetation. This should include any weeds, grasses, shrubs, and trees. The actual extent of site stripping should be determined in the field by the geotechnical engineer, based on the organic content and stability of the materials encountered. This material should be removed and disposed of off-site.

Treatment of Existing Soils: Building Pads

Remedial grading should be performed within the proposed building areas in order to remove the potentially collapsible native soils. Based on conditions encountered at the boring locations, these materials extend to depths of 2 to 4± feet. To provide a uniform blanket of structural fill beneath the foundations and floor slabs for the new structure, it is recommended that the existing soils within the proposed building areas be overexcavated to a depth of at least 3 feet below proposed building pad subgrade elevation and to a depth of at least 3 feet below existing grade, whichever is greater.

Where not encompassed within the general building pad overexcavation, additional overexcavation should be performed within the influence zones of the new foundations, to provide for a new layer of compacted structural fill extending to a depth of 2 feet below proposed bearing grade.

The overexcavation areas should extend at least 5 feet beyond the building perimeters, and to an extent equal to the depth of fill below the new foundations. If the proposed structure incorporates any exterior columns (such as for a canopy or overhang) the overexcavation should also encompass these areas.

Following completion of the overexcavation, the subgrade soils within the building areas should be evaluated by the geotechnical engineer to verify their suitability to serve as the structural fill subgrade, as well as to support the foundation loads of the new structures. This evaluation should include proofrolling and probing to identify any soft, loose or otherwise unstable soils that must be removed. Some localized areas of deeper excavation may be required if additional fill materials or loose, porous, or low density native soils are encountered at the base of the overexcavation.

After a suitable overexcavation subgrade has been achieved, the exposed soils should be scarified to a depth of at least 12 inches, moisture treated to 2 to 4 percent above optimum moisture content. The subgrade soils should then be recompacted to at least 90 percent of the ASTM D-1557 maximum dry density. The previously excavated soils may then be replaced as compacted structural fill.

Treatment of Existing Soils: Retaining Walls and Site Walls

The existing soils within the areas of any proposed retaining walls should be overexcavated to a depth of 2 feet below foundation bearing grade and replaced as compacted structural fill as discussed above for the proposed building pads. Additional overexcavation should be performed to remove any existing fill soils. The foundation areas for non-retaining site walls should be overexcavated to a depth of 1 foot below proposed foundation bearing grade. The overexcavation subgrade soils should be evaluated by the geotechnical engineer prior to scarifying, moisture conditioning, and recompacting the upper 12 inches of exposed subgrade soils. The previously excavated soils may then be replaced as compacted structural fill.

Treatment of Existing Soils: Parking and Drive Areas

Based on economic considerations, overexcavation of the existing soils in the new parking and drive areas is not considered warranted, with the exception of areas where lower strength, or unstable, soils are identified by the geotechnical engineer during grading. Subgrade preparation in the new parking and drive areas should initially consist of removal of all soils disturbed during stripping and demolition operations.

The geotechnical engineer should then evaluate the subgrade to identify any areas of additional unsuitable soils. Any such materials should be removed to a level of firm and unyielding soil. The exposed subgrade soils should then be scarified to a depth of 12± inches, moisture conditioned to at least 2 to 4 percent above optimum, and recompacted to at least 90 percent of the ASTM D-1557 maximum dry density. Based on the presence of variable strength surficial soils throughout the site, it is expected that some isolated areas of additional overexcavation may be required to remove zones of lower strength, unsuitable soils.

The grading recommendations presented above for the proposed parking area assume that the owner and/or developer can tolerate minor amounts of settlement within the proposed parking areas. The grading recommendations presented above do not completely mitigate the extent of undocumented fill soils or collapsible native alluvium in the parking areas. As such, settlement and associated pavement distress could occur. Typically, repair of such distressed areas involves significantly lower costs than completely mitigating these soils at the time of construction. If the owner cannot tolerate the risk of such settlements, the parking area should be graded in a manner similar to that described for the building areas.

Fill Placement

- Fill soils should be placed in thin (6± inches), near-horizontal lifts, moisture conditioned to 2 to 4 percent above the optimum moisture content, and compacted.
- On-site soils may be used for fill provided they are cleaned of any debris to the satisfaction of the geotechnical engineer.
- All grading and fill placement activities should be completed in accordance with the requirements of the 2010 CBC and the grading code of the City of Moreno Valley.
- All fill soils should be compacted to at least 90 percent of the ASTM D-1557 maximum dry density. Fill soils should be well mixed.
- Compaction tests should be performed periodically by the geotechnical engineer as random verification of compaction and moisture content. These tests are intended to aid the contractor. Since the tests are taken at discrete locations and depths, they may not be indicative of the entire fill and therefore should not relieve the contractor of his responsibility to meet the job specifications.

Imported Structural Fill

All imported structural fill should consist of very low expansive (EI < 20), well graded soils possessing at least 10 percent fines (that portion of the sample passing the No. 200 sieve). Additional specifications for structural fill are presented in the Grading Guide Specifications, included as Appendix D.

Utility Trench Backfill

In general, all utility trench backfill soils should be compacted to at least 90 percent of the ASTM D-1557 maximum dry density. It is recommended that materials in excess of 3 inches in size not be used for utility trench backfill. Compacted trench backfill should conform to the requirements of the local grading code, and more restrictive requirements may be indicated by City of Moreno Valley. All utility trench backfills should be witnessed by the geotechnical engineer. The trench backfill soils should be compaction tested where possible; probed and visually evaluated elsewhere.

Utility trenches which parallel a footing, and extending below a 1h:1v plane projected from the outside edge of the footing should be backfilled with structural fill soils, compacted to at least 90 percent of the ASTM D-1557 standard. Pea gravel backfill should not be used for these trenches.

6.4 Construction Considerations

Excavation Considerations

The near-surface soils generally consist of sandy clays, silty clays, and clayey sands. Some of these materials may be subject to minor caving within shallow excavations. Where caving does occur, flattened excavation slopes may be sufficient to provide excavation stability. On a preliminary basis, the inclination of temporary slopes should not exceed 1.5h:1v. Deeper excavations may require some form of external stabilization such as shoring or bracing. Maintaining adequate moisture content within the near-surface soils will improve excavation stability. All excavation activities on this site should be conducted in accordance with Cal-OSHA regulations.

Moisture Sensitive Subgrade Soils

Most of the near surface soils possess appreciable silt and clay content and may become unstable if exposed to significant moisture infiltration or disturbance by construction traffic. In addition, based on their granular content, some of the on-site soils will also be susceptible to erosion. The site should, therefore, be graded to prevent ponding of surface water and to prevent water from running into excavations.

Expansive Soils

The near surface on-site soils have been determined to possess a low to medium expansion potential. Therefore, care should be given to proper moisture conditioning of all building pad subgrade soils to a moisture content of 2 to 4 percent above the Modified Proctor optimum during site grading. All imported fill soils should have very low expansive (EI < 20) characteristics. **In addition to adequately moisture conditioning the subgrade soils and fill soils during grading, special care must be taken to maintain moisture content of these soils at 2 to 4 percent above the Modified Proctor optimum. This will require the contractor to frequently moisture condition these soils throughout the grading process, unless grading occurs during a period of relatively wet weather**.

Due to the presence of expansive soils at this site, provisions should be made to limit the potential for surface water to penetrate the soils immediately adjacent to the structures. These provisions should include directing surface runoff into rain gutters and area drains, reducing the extent of landscaped areas around the structure, and sloping the ground surface away from the buildings. Where possible, it is recommended that landscaped planters not be located immediately adjacent to the proposed buildings. If landscaped planters around the buildings are necessary, it is recommended that drought tolerant plants or a drip irrigation system be utilized, to minimize the potential for deep moisture penetration around the structure. Other provisions, as determined by the civil engineer may also be appropriate.

Corrosion Potential

The results of the electrical resistivity and pH testing indicate that the tested sample of the on-site soils has a resistivity value of 1,270 ohm-cm and a pH value of 7.6. These test results have been evaluated in accordance with guidelines published by the Ductile Iron Pipe Research Association (DIPRA). The DIPRA guidelines consist of a point system by which characteristics of the soils are used to quantify the corrosivity characteristics of the site. Resistivity and pH are two of the five factors that enter into the evaluation procedure. Relative soil moisture content as well as redox potential and sulfides are also included. We have evaluated the corrosivity characteristics of the on-site soils using resistivity, pH and moisture content. **Based on these factors, and utilizing the DIPRA procedure, some of the on-site soils are considered to be severely corrosive to ductile iron pipe.** Therefore, it is expected that polyethylene encasement will be required for ductile iron pipe. The client may also wish to contact a corrosion engineer to provide a more thorough evaluation.

Groundwater

The static groundwater table at this site is considered to exist at a depth in excess of 30± feet. Therefore, groundwater is not expected to impact grading or foundation construction activities.

6.5 Foundation Design and Construction

Based on the preceding grading recommendations, it is assumed that the new building pads will be underlain by structural fill soils used to replace the potentially collapsible near-surface alluvial soils. These new structural fill soils are expected to extend to depths of at least 2 feet below proposed foundation bearing grade, underlain by 1± foot of additional soil that has been densified and moisture conditioned in place. Based on this subsurface profile, the proposed structure may be supported on conventional shallow foundations.

Foundation Design Parameters

New square and rectangular footings may be designed as follows:

- Maximum, net allowable soil bearing pressure: 2,500 lbs/ft^2.
- Minimum wall/column footing width: 14 inches/24 inches.
- Minimum longitudinal steel reinforcement within strip footings: Four (4) No. 5 rebars (2 top and 2 bottom), due to the presence of low to medium expansive soils.
- Minimum foundation embedment: 12 inches into suitable structural fill soils, and at least 24 inches below adjacent exterior grade. Interior column footings may be placed immediately beneath the floor slabs.
- It is recommended that the perimeter building foundations be continuous across all exterior doorways. Any flatwork adjacent to the exterior doors should be doweled into the perimeter foundations in a manner determined by the structural engineer.

The allowable bearing pressures presented above may be increased by 1/3 when considering short duration wind or seismic loads. The minimum steel reinforcement recommended above is based on geotechnical considerations; additional reinforcement may be necessary for structural considerations. The actual design of the foundations should be determined by the structural engineer.

Foundation Construction

The foundation subgrade soils should be evaluated at the time of overexcavation, as discussed in Section 6.3 of this report. It is further recommended that the foundation subgrade soils be evaluated by the geotechnical engineer immediately prior to steel or concrete placement. Soils suitable for direct foundation support should consist of newly placed structural fill, compacted to at least 90 percent of the ASTM D-1557 maximum dry density. Any unsuitable materials should be removed to a depth of suitable bearing compacted structural fill, with the resulting excavations backfilled with compacted fill soils. As an alternative, lean concrete slurry (500 to 1,500 psi) may be used to backfill such isolated overexcavations.

The foundation subgrade soils should also be properly moisture conditioned to at least 2 to 4 percent of the Modified Proctor optimum, to a depth of at least 12 inches below bearing grade. **Since it is typically not feasible to increase the moisture content of the floor slabs and foundation subgrade soils once rough grading has been completed, care should be taken to maintain the moisture content of the building pad subgrade soils throughout the construction process.**

Estimated Foundation Settlements

Post-construction total and differential settlements of shallow foundations designed and constructed in accordance with the previously presented recommendations are estimated to be less than 1.0 and 0.5 inches, respectively. Differential movements are expected to occur over a 30-foot span, thereby resulting in an angular distortion of less than 0.002 inches per inch.

Lateral Load Resistance

Lateral load resistance will be developed by a combination of friction acting at the base of foundations and slabs and the passive earth pressure developed by footings below grade. The following friction and passive pressure may be used to resist lateral forces:

- Passive Earth Pressure: 275 lbs/ft^3
- Friction Coefficient: 0.28

These are allowable values, and include a factor of safety. When combining friction and passive resistance, the passive pressure component should be reduced by one-third. These values assume that footings will be poured directly against compacted structural fill. The maximum allowable passive pressure is 2500 lbs/ft^2.

6.6 Floor Slab Design and Construction

Subgrades which will support new floor slabs should be prepared in accordance with the recommendations contained in the ***Site Grading Recommendations*** section of this report. Based on the anticipated grading which will occur at this site, the floor of the new structure may be constructed as a conventional slab-on-grade supported on newly placed structural fill, extending to a depth of at least 3 feet below existing grade. Based on geotechnical considerations, the floor slabs may be designed as follows:

- Minimum slab thickness: 5 inches.

- Minimum slab reinforcement: No. 3 bars at 18-inches on center, in both directions due to the presence of low to medium expansive soils. The actual floor slab reinforcement should be determined by the structural engineer, based upon the imposed loading.

- Slab underlayment: Slab underlayment: If moisture sensitive floor coverings will be used then minimum slab underlayment should consist of a moisture vapor barrier constructed below these areas. The moisture vapor barrier should meet or exceed the Class A rating as defined by ASTM E 1745-97 and have a permeance rating less than 0.01 perms as described in ASTM E 96-95 and ASTM E 154-88. The moisture vapor barrier should be properly constructed in accordance with all applicable manufacturer specifications. Given that a rock free subgrade is anticipated and that a capillary break is not required, sand below the barrier is not required. The need for sand and/or the amount of sand above the moisture vapor barrier should be specified by the structural engineer or concrete contractor. The selection of sand above the barrier is not a geotechnical engineering issue and hence outside our purview. Where moisture sensitive floor coverings are not anticipated, the vapor barrier may be eliminated.

- Moisture condition the floor slab subgrade soils to 2 to 4 percent above the Modified Proctor optimum moisture content, to a depth of 12 inches. The moisture content of the floor slab subgrade soils should be verified by the geotechnical engineer within 24 hours prior to concrete placement.

- Proper concrete curing techniques should be utilized to reduce the potential for slab curling or the formation of excessive shrinkage cracks.

The actual design of the floor slabs should be completed by the structural engineer to verify adequate thickness and reinforcement.

6.7 Trash Enclosure Design Parameters

It is anticipated that trash enclosures will be constructed at the subject site. It is expected that the trash enclosures as well as the approach slabs will be subjected to relatively heavy wheel loads imposed by trash removal equipment.

The subgrade soils in the area of the trash enclosures and the approach slabs should be prepared in accordance with the recommendations for the parking areas, presented in Section 6.3 of this report. As such, it is expected that the trash enclosures will be underlain by structural fill soils, extending to a depth of 1 foot below proposed subgrade elevation. Based on geotechnical considerations, the following recommendations are provided for the design of the trash enclosures and the trash enclosure approach slabs:

- The trash enclosure may consist of a 6-inch thick concrete slab incorporating a perimeter footing or a turned down edge, extending to a depth of at least 12 inches below adjacent finished grade. If the trash enclosure will incorporate rigid walls such as masonry block or tilt-up concrete, the perimeter foundations should be designed in accordance with the recommendations previously presented in Section 6.5 of this report.

- Reinforcement within the trash enclosure slab should consist of at least No. 3 bars at 18-inches on-center, in both directions.

- The trash enclosure approach slab should be constructed of Portland cement concrete, at least 6 inches in thickness. Reinforcement within the approach slab should consist of at least No. 3 bars at 18-inches on-center, in both directions.

- The trash enclosure and approach slab subgrades should be moisture conditioned to 2 to 4 percent above the optimum moisture content to a depth of 12 inches. The trash enclosure slab and the approach slab should be structurally connected, to reduce the potential for differential movement between the two slabs.

- The actual design of the trash enclosure and the trash enclosure approach slab should be completed by the structural engineer to verify adequate thickness and reinforcement.

6.8 Retaining Wall Design and Construction

Although not indicated on the site plan, some small retaining walls may be required to facilitate the new site grades. It is also expected that some retaining walls will be required in the truck loading dock areas. All of these walls are expected to be less than 3 to 5± feet in height. The parameters recommended for use in the design of these walls are presented below.

Retaining Wall Design Parameters

Based on the soil conditions encountered at the boring locations, the following parameters may be used in the design of new retaining walls for this site. The on-site soils generally consist of sandy clays, silty clays, clayey sands, and silty sands. **It is recommended that the on-site expansive clayey soils not be used to backfill any proposed on-site retaining walls.** Based on the composition of the on-site sands, these soils have been assigned a friction angle of 28 degrees.

In order to use the design parameters for the imported select fill, this material must be placed within the entire active failure wedge. This wedge is defined as extending from the heel of the retaining wall upwards at an angle of approximately 60 degrees.

If desired, SCG could provide design parameters for an alternative select backfill material behind the retaining walls. The use of select backfill material could result in lower lateral earth pressures. In order to use the design parameters for the imported select fill, this material must be placed within the entire active failure wedge. This wedge is defined as extending from the heel of the retaining wall upwards at an angle of approximately 60° from horizontal. If select backfill material behind the retaining wall is desired, SCG should be contacted for supplementary recommendations.

RETAINING WALL DESIGN PARAMETERS

Design Parameter		Soil Type: On-Site Sands and Clayey Sands
Internal Friction Angle (ϕ)		30°
Unit Weight		125 lbs/ft^3
Equivalent Fluid Pressure:	Active Condition (level backfill)	43 lbs/ft^3
	Active Condition (2h:1v backfill)	76 lbs/ft^3
	At-Rest Condition (level backfill)	63 lbs/ft^3

Regardless of the backfill type, the walls should be designed using a soil-footing coefficient of friction of 0.28 and an equivalent passive pressure of 275 lbs/ft^3. The structural engineer should incorporate appropriate factors of safety in the design of the retaining walls.

The active earth pressure may be used for the design of retaining walls that do not directly support structures or support soils that in turn support structures and which will be allowed to deflect. The at-rest earth pressure should be used for walls that will not be allowed to deflect such as those which will support foundation bearing soils, or which will support foundation loads directly.

Where the soils on the toe side of the retaining wall are not covered by a "hard" surface such as a structure or pavement, the upper 1 foot of soil should be neglected when calculating passive resistance due to the potential for the material to become disturbed or degraded during the life of the structure.

Retaining Wall Foundation Design

The retaining wall foundations should be supported within newly placed compacted structural fill, extending to a depth of at least 2 feet below the proposed bearing grade. Foundations to support new retaining walls should be designed in accordance with the general Foundation Design Parameters presented in a previous section of this report.

Seismic Lateral Earth Pressures

In accordance with the 2013 CBC, any retaining walls more than 6 feet in height must be designed for seismic lateral earth pressures. If walls 6 feet or more are required for this site, the geotechnical engineer should be contacted for supplementary seismic lateral earth pressure recommendations.

Backfill Material

It is recommended that a properly installed prefabricated drainage composite such as the MiraDRAIN 6000XL (or approved equivalent), which is specifically designed for use behind retaining walls, may be used. If the layer of free-draining material is not covered by an impermeable surface, such as a structure or pavement, a 12-inch thick layer of a low permeability soil should be placed over the backfill to reduce surface water migration to the underlying soils. The layer of free draining granular material should be separated from the backfill soils by a suitable geotextile, approved by the geotechnical engineer

All retaining wall backfill should be placed and compacted under engineering controlled conditions in the necessary layer thicknesses to ensure an in-place density between 90 and 93 percent of the maximum dry density as determined by the Modified Proctor test (ASTM D1557-91). Care should be taken to avoid over-compaction of the soils behind the retaining walls, and the use of heavy compaction equipment should be avoided.

Subsurface Drainage

As previously indicated, the retaining wall design parameters are based upon drained backfill conditions. Consequently, some form of permanent drainage system will be necessary in conjunction with the appropriate backfill material. Subsurface drainage may consist of either:

- A weep hole drainage system typically consisting of a series of 4-inch diameter holes in the wall situated slightly above the ground surface elevation on the exposed side of the wall and at an approximate 8-foot on-center spacing. The weep holes should include a 2 cubic foot pocket of open graded gravel, surrounded by an approved geotextile fabric, at each weep hole location.

- A 4-inch diameter perforated pipe surrounded by 2 cubic feet of gravel per linear foot of drain placed behind the wall, above the retaining wall footing. The gravel layer should be wrapped in a suitable geotextile fabric to reduce the potential for migration of fines. The footing drain should be extended to daylight or tied into a storm drainage system.

6.9 Pavement Design Parameters

Site preparation in the pavement area should be completed as previously recommended in the ***Site Grading Recommendations*** section of this report. The subsequent pavement recommendations assume proper drainage and construction monitoring, and are based on either PCA or CALTRANS design parameters for a twenty (20) year design period. However, these designs also assume a routine pavement maintenance program to obtain the anticipated 20-year pavement service life.

Pavement Subgrades

It is anticipated that the new pavements will be primarily supported on recompacted native alluvial soils that consist of fine sandy clays and clayey fine sands. These existing soils are considered to possess fair pavement support characteristics. Based on their classification, these materials are expected to possess good pavement support characteristics, with R-values in the range of 35 to 45. Since R-value was not included in the scope of services for this project, the subsequent pavement design is based upon an assumed R-value of 35. Any fill material imported to the site should have support characteristics equal to or greater than that of the on-site soils and be placed and compacted under engineering controlled conditions. It is recommended that R-value testing be performed after completion of rough grading. Depending upon the results of the R-value testing, it may be feasible to use thinner pavement sections in some areas of the site.

Asphaltic Concrete

Presented below are the recommended thicknesses for new flexible pavement structures consisting of asphaltic concrete over a granular base. The pavement designs are based on the traffic indices (TI's) indicated. The client and/or civil engineer should verify that these TI's are representative of the anticipated traffic volumes. If the client and/or civil engineer determine that the expected traffic volume will exceed the applicable traffic index, we should be contacted for supplementary recommendations. The design traffic indices equate to the following approximate daily traffic volumes over a 20 year design life, assuming six operational traffic days per week.

Traffic Index	**No. of Heavy Trucks per Day**
4.0	0
5.0	1
6.0	3
7.0	11
8.0	35
9.0	93

For the purpose of the traffic volumes indicated above, a truck is defined as a 5-axle tractor trailer unit with one 8-kip axle and two 32-kip tandem axles. All of the traffic indices allow for 1,000 automobiles per day.

ASPHALT PAVEMENTS (R=35)					
Materials	**Thickness (inches)**				
	Auto Parking and Auto Drive Lanes (TI = 4.0 to 5.0)	Truck Traffic			
		TI = 6.0	TI = 7.0	TI = 8.0	TI = 9.0
Asphalt Concrete	3	3½	4	5	5½
Aggregate Base	5	7	9	10	11
Compacted Subgrade	12	12	12	12	12

The aggregate base course should be compacted to at least 95 percent of the ASTM D-1557 maximum dry density. The asphaltic concrete should be compacted to at least 95 percent of the

Marshall maximum density, as determined by ASTM D-2726. The aggregate base course may consist of crushed aggregate base (CAB) or crushed miscellaneous base (CMB), which is a recycled gravel, asphalt and concrete material. The gradation, R-Value, Sand Equivalent, and Percentage Wear of the CAB or CMB should comply with appropriate specifications contained in the current edition of the "Greenbook" Standard Specifications for Public Works Construction.

Portland Cement Concrete

The preparation of the subgrade soils within concrete pavement areas should be performed as previously described for proposed asphalt pavement areas. The minimum recommended thicknesses for the Portland Cement Concrete pavement sections are as follows:

PORTLAND CEMENT CONCRETE PAVEMENTS (R=35)				
Materials	**Thickness (inches)**			
	Autos and Light Truck Traffic (TI = 6.0)	Truck Traffic		
		TI = 7.0	TI = 8.0	TI = 9.0
PCC	5	6½	8	9
Compacted Subgrade (95% minimum compaction)	12	12	12	12

The concrete should have a 28-day compressive strength of at least 3,000 psi. The maximum joint spacing within all of the PCC pavements is recommended to be equal to or less than 30 times the pavement thickness.

7.0 GENERAL COMMENTS

This report has been prepared as an instrument of service for use by the client, in order to aid in the evaluation of this property and to assist the architects and engineers in the design and preparation of the project plans and specifications. This report may be provided to the contractor(s) and other design consultants to disclose information relative to the project. However, this report is not intended to be utilized as a specification in and of itself, without appropriate interpretation by the project architect, civil engineer, and/or structural engineer. The reproduction and distribution of this report must be authorized by the client and Southern California Geotechnical, Inc. Furthermore, any reliance on this report by an unauthorized third party is at such party's sole risk, and we accept no responsibility for damage or loss which may occur. The client(s)' reliance upon this report is subject to the Engineering Services Agreement, incorporated into our proposal for this project.

The analysis of this site was based on a subsurface profile interpolated from limited discrete soil samples. While the materials encountered in the project area are considered to be representative of the total area, some variations should be expected between boring locations and sample depths. If the conditions encountered during construction vary significantly from those detailed herein, we should be contacted immediately to determine if the conditions alter the recommendations contained herein.

This report has been based on assumed or provided characteristics of the proposed development. It is recommended that the owner, client, architect, structural engineer, and civil engineer carefully review these assumptions to ensure that they are consistent with the characteristics of the proposed development. If discrepancies exist, they should be brought to our attention to verify that they do not affect the conclusions and recommendations contained herein. We also recommend that the project plans and specifications be submitted to our office for review to verify that our recommendations have been correctly interpreted.

The analysis, conclusions, and recommendations contained within this report have been promulgated in accordance with generally accepted professional geotechnical engineering practice. No other warranty is implied or expressed.

APPENDIX A

SOURCE: RIVERSIDE COUNTY
THOMAS GUIDE, 2013

SITE LOCATION MAP

PROPOSED MORENO VALLEY LOGISTICS CENTER

MORENO VALLEY, CALIFORNIA

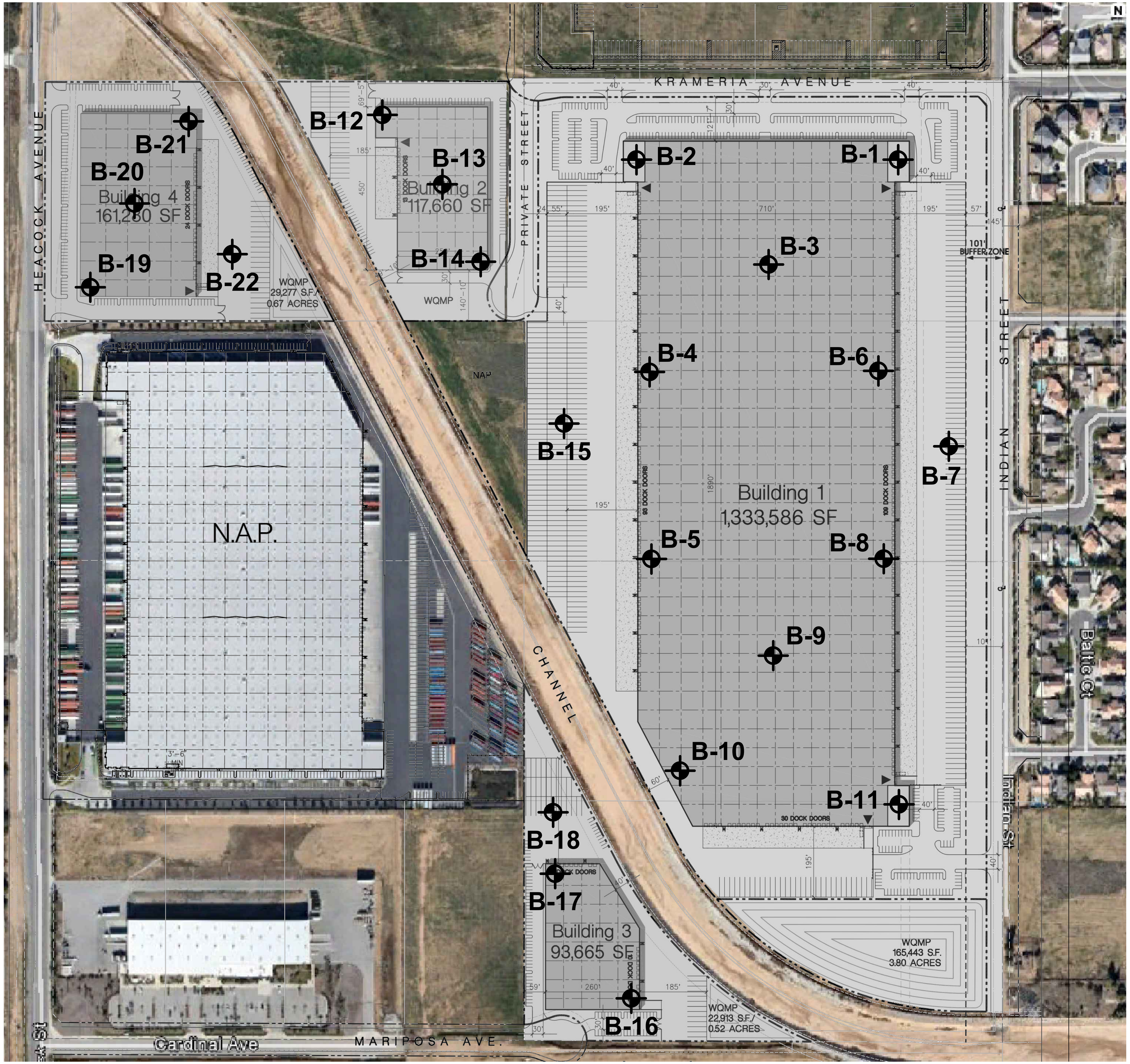
SCALE: 1" = 2400'

DRAWN: PM
CHKD: JAS

SCG PROJECT
14G160-1R

PLATE 1

SoCalGeo
SOUTHERN CALIFORNIA GEOTECHNICAL



GEOTECHNICAL LEGEND

APPROXIMATE BORING LOCATION

BORING LOCATION PLAN

PROPOSED MORENO VALLEY LOGISTICS CENTER

MORENO VALLEY, CALIFORNIA

SCALE: 1" = 300'

DRAWN: PM
CHKD: JAS

SCG PROJECT
14G160-1R

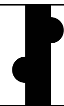
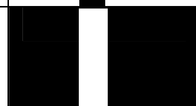
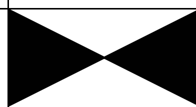
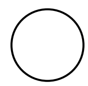
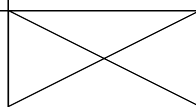
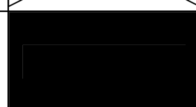
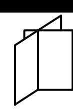
PLATE 2

SoCalGeo

SOUTHERN CALIFORNIA GEOTECHNICAL

APPENDIX B

BORING LOG LEGEND

SAMPLE TYPE	GRAPHICAL SYMBOL	SAMPLE DESCRIPTION
AUGER		SAMPLE COLLECTED FROM AUGER CUTTINGS, NO FIELD MEASUREMENT OF SOIL STRENGTH. (DISTURBED)
CORE		ROCK CORE SAMPLE: TYPICALLY TAKEN WITH A DIAMOND-TIPPED CORE BARREL. TYPICALLY USED ONLY IN HIGHLY CONSOLIDATED BEDROCK.
GRAB		SOIL SAMPLE TAKEN WITH NO SPECIALIZED EQUIPMENT, SUCH AS FROM A STOCKPILE OR THE GROUND SURFACE. (DISTURBED)
CS		CALIFORNIA SAMPLER: 2-1/2 INCH I.D. SPLIT BARREL SAMPLER, LINED WITH 1-INCH HIGH BRASS RINGS. DRIVEN WITH SPT HAMMER. (RELATIVELY UNDISTURBED)
NSR		NO RECOVERY: THE SAMPLING ATTEMPT DID NOT RESULT IN RECOVERY OF ANY SIGNIFICANT SOIL OR ROCK MATERIAL.
SPT		STANDARD PENETRATION TEST: SAMPLER IS A 1.4 INCH INSIDE DIAMETER SPLIT BARREL, DRIVEN 18 INCHES WITH THE SPT HAMMER. (DISTURBED)
SH		SHELBY TUBE: TAKEN WITH A THIN WALL SAMPLE TUBE, PUSHED INTO THE SOIL AND THEN EXTRACTED. (UNDISTURBED)
VANE		VANE SHEAR TEST: SOIL STRENGTH OBTAINED USING A 4 BLADED SHEAR DEVICE. TYPICALLY USED IN SOFT CLAYS-NO SAMPLE RECOVERED.

COLUMN DESCRIPTIONS

DEPTH: Distance in feet below the ground surface.

SAMPLE: Sample Type as depicted above.

BLOW COUNT: Number of blows required to advance the sampler 12 inches using a 140 lb hammer with a 30-inch drop. 50/3" indicates penetration refusal (>50 blows) at 3 inches. WH indicates that the weight of the hammer was sufficient to push the sampler 6 inches or more.

POCKET PEN.: Approximate shear strength of a cohesive soil sample as measured by pocket penetrometer.

GRAPHIC LOG: Graphic Soil Symbol as depicted on the following page.

DRY DENSITY: Dry density of an undisturbed or relatively undisturbed sample in lbs/ft^3.

MOISTURE CONTENT: Moisture content of a soil sample, expressed as a percentage of the dry weight.

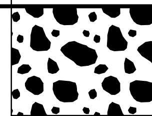
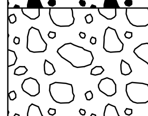
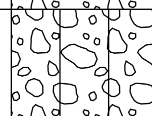
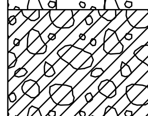
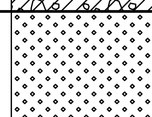
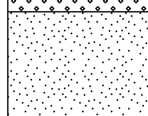
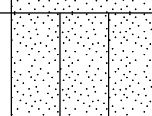
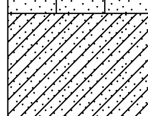
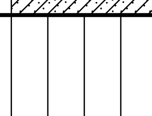
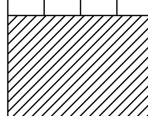
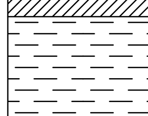
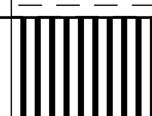
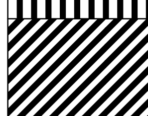
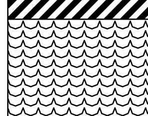
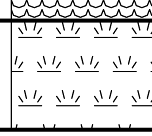
LIQUID LIMIT: The moisture content above which a soil behaves as a liquid.

PLASTIC LIMIT: The moisture content above which a soil behaves as a plastic.

PASSING #200 SIEVE: The percentage of the sample finer than the #200 standard sieve.

UNCONFINED SHEAR: The shear strength of a cohesive soil sample, as measured in the unconfined state.

SOIL CLASSIFICATION CHART

MAJOR DIVISIONS			SYMBOLS GRAPH	SYMBOLS LETTER	TYPICAL DESCRIPTIONS
COARSE GRAINED SOILS	GRAVEL AND GRAVELLY SOILS	CLEAN GRAVELS		GW	WELL-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
		(LITTLE OR NO FINES)		GP	POORLY-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES
	MORE THAN 50% OF COARSE FRACTION RETAINED ON NO. 4 SIEVE	GRAVELS WITH FINES		GM	SILTY GRAVELS, GRAVEL - SAND - SILT MIXTURES
		(APPRECIABLE AMOUNT OF FINES)		GC	CLAYEY GRAVELS, GRAVEL - SAND - CLAY MIXTURES
MORE THAN 50% OF MATERIAL IS LARGER THAN NO. 200 SIEVE SIZE	SAND AND SANDY SOILS	CLEAN SANDS		SW	WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES
		(LITTLE OR NO FINES)		SP	POORLY-GRADED SANDS, GRAVELLY SAND, LITTLE OR NO FINES
	MORE THAN 50% OF COARSE FRACTION PASSING ON NO. 4 SIEVE	SANDS WITH FINES		SM	SILTY SANDS, SAND - SILT MIXTURES
		(APPRECIABLE AMOUNT OF FINES)		SC	CLAYEY SANDS, SAND - CLAY MIXTURES
FINE GRAINED SOILS	SILTS AND CLAYS	LIQUID LIMIT LESS THAN 50		ML	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS OR CLAYEY SILTS WITH SLIGHT PLASTICITY
				CL	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAN CLAYS
				OL	ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY
MORE THAN 50% OF MATERIAL IS SMALLER THAN NO. 200 SIEVE SIZE	SILTS AND CLAYS	LIQUID LIMIT GREATER THAN 50		MH	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SAND OR SILTY SOILS
				CH	INORGANIC CLAYS OF HIGH PLASTICITY
				OH	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS
HIGHLY ORGANIC SOILS				PT	PEAT, HUMUS, SWAMP SOILS WITH HIGH ORGANIC CONTENTS

NOTE: DUAL SYMBOLS ARE USED TO INDICATE BORDERLINE SOIL CLASSIFICATIONS

SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

BORING NO. B-1

JOB NO.: 14G160 | DRILLING DATE: 6/24/14 | WATER DEPTH: Dry
PROJECT: Prop. Moreno Valley Logistics Center | DRILLING METHOD: Hollow Stem Auger | CAVE DEPTH: 18 feet
LOCATION: Moreno Valley, California | LOGGED BY: Eric Torres | READING TAKEN: At Completion

FIELD RESULTS						LABORATORY RESULTS						
DEPTH (FEET)	SAMPLE	BLOW COUNT	POCKET PEN. (TSF)	GRAPHIC LOG	DESCRIPTION SURFACE ELEVATION: --- MSL	DRY DENSITY (PCF)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PASSING #200 SIEVE (%)	UNCONFINED SHEAR (TSF)	COMMENTS
	X	12			<u>ALLUVIUM:</u> Brown Silty fine Sand, trace medium Sand, trace to little Clay, slightly porous, loose to medium dense-dry to damp		3					
	X	9					5					
5												
	X	15			Brown Clayey fine Sand, trace Silt, medium dense-damp to moist		9					
	X	21	3.5		Dark Brown fine Sandy Clay to Clayey fine Sand, medium dense to very stiff-moist		13					
10												
	X	29	3.5				11					
					Brown Silty fine to medium Sand, medium dense-damp to moist		7					
15												
					Brown fine Sandy Silt, little Clay, stiff to very stiff-moist to very moist							
	X	17					19					
20												
	X	13					17					
25					Boring Terminated at 25'							

SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

BORING NO. B-2

JOB NO.: 14G160 | DRILLING DATE: 6/24/14 | WATER DEPTH: Dry
PROJECT: Prop. Moreno Valley Logistics Center | DRILLING METHOD: Hollow Stem Auger | CAVE DEPTH: 18 feet
LOCATION: Moreno Valley, California | LOGGED BY: Eric Torres | READING TAKEN: At Completion

FIELD RESULTS						LABORATORY RESULTS						
DEPTH (FEET)	SAMPLE	BLOW COUNT	POCKET PEN. (TSF)	GRAPHIC LOG	DESCRIPTION SURFACE ELEVATION: --- MSL	DRY DENSITY (PCF)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PASSING #200 SIEVE (%)	UNCONFINED SHEAR (TSF)	COMMENTS
		26			ALLUVIUM: Brown Silty fine Sand, trace medium Sand, medium dense-dry to damp	105	2					
		11			Brown Silty fine to medium Sand, loose-dry to damp	108	3					
5		38				123	3					
					Brown Clayey fine Sand, little to some Silt, trace medium Sand, dense-damp							
		26	4.5+		Dark Brown fine Sandy Clay, trace to little Silt, slightly porous, very stiff-damp to moist	122	9					
10		26	4.5+			115	19					
15		25	4.5+		@ 13½ to 15 feet, trace calcareous veining		14					
					Dark Brown Silty fine Sand, little to some Clay, slightly porous, medium dense-damp to moist							
20		16					8					
					Brown Clayey fine Sand, little Silt, medium dense-moist							
25		20					12					
					Dark Brown Silty fine Sand, little to some Clay, medium dense-moist							
30		23					11					
					Boring Terminated at 30'							

SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

BORING NO. B-3

JOB NO.: 14G160
PROJECT: Prop. Moreno Valley Logistics Center
LOCATION: Moreno Valley, California
DRILLING DATE: 6/24/14
DRILLING METHOD: Hollow Stem Auger
LOGGED BY: Eric Torres
WATER DEPTH: Dry
CAVE DEPTH: 18 feet
READING TAKEN: At Completion

FIELD RESULTS						LABORATORY RESULTS						
DEPTH (FEET)	SAMPLE	BLOW COUNT	POCKET PEN. (TSF)	GRAPHIC LOG	DESCRIPTION SURFACE ELEVATION: --- MSL	DRY DENSITY (PCF)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PASSING #200 SIEVE (%)	UNCONFINED SHEAR (TSF)	COMMENTS
					ALLUVIUM: Brown Silty fine Sand, trace Clay, medium dense-dry to damp							
	X	23				88	2					
					Dark Brown Clayey Silt, little to some fine Sand, slightly porous, stiff-dry to damp							
					Brown Clayey fine Sand, little to some Silt, trace calcareous veining, medium dense-damp to moist							
	X	15				105	7					
5	X	30				113	7					
					Dark Brown Silty fine to medium Sand, trace to little Clay, medium dense-damp to moist							
	X	18				111	4					
	X	33				130	7					
10												
					Brown Clayey fine Sand, medium dense-damp to moist							
	X	29					8					
15												
					Boring Terminated at 15'							

TBL 14G160.GPJ SOCALGEO.GDT 3/24/15

SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

BORING NO. B-4

JOB NO.: 14G160
PROJECT: Prop. Moreno Valley Logistics Center
LOCATION: Moreno Valley, California

DRILLING DATE: 6/24/14
DRILLING METHOD: Hollow Stem Auger
LOGGED BY: Eric Torres

WATER DEPTH: Dry
CAVE DEPTH: 18 feet
READING TAKEN: At Completion

FIELD RESULTS						LABORATORY RESULTS						
DEPTH (FEET)	SAMPLE	BLOW COUNT	POCKET PEN. (TSF)	GRAPHIC LOG	DESCRIPTION SURFACE ELEVATION: --- MSL	DRY DENSITY (PCF)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PASSING #200 SIEVE (%)	UNCONFINED SHEAR (TSF)	COMMENTS
	X	26	4.5+		ALLUVIUM: Dark Brown Silty Clay, trace calcareous veining, very stiff-dry to damp		8					EI = 66 @ 0 to 5 feet
5	X	15	3.0		Brown Clayey Silt, little fine Sand, trace calcareous veining, stiff to very stiff-damp to moist		13					
	X	14			Dark Brown Silty fine Sand, little Clay, medium dense-damp to moist		10					
10	X	24			Dark Brown Clayey fine Sand, little to some Silt, medium dense-damp to moist		6					
15	X	24			Dark Brown fine to medium Sand, trace Silt, medium dense-damp to moist		10 7					
20	X	37	4.5+		Brown Silty Clay, hard-damp		10					
					Boring Terminated at 20'							

TBL 14G160.GPJ SOCALGEO.GDT 3/24/15

SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

BORING NO. B-5

JOB NO.: 14G160
PROJECT: Prop. Moreno Valley Logistics Center
LOCATION: Moreno Valley, California

DRILLING DATE: 6/24/14
DRILLING METHOD: Hollow Stem Auger
LOGGED BY: Eric Torres

WATER DEPTH: Dry
CAVE DEPTH: 18 feet
READING TAKEN: At Completion

SURFACE ELEVATION: --- MSL

DEPTH (FEET)	SAMPLE	BLOW COUNT	POCKET PEN. (TSF)	GRAPHIC LOG	DESCRIPTION	DRY DENSITY (PCF)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PASSING #200 SIEVE (%)	UNCONFINED SHEAR (TSF)	COMMENTS
					ALLUVIUM: Brown Silty fine to medium Sand, medium dense-dry to damp							
	X	16					2					
					Brown fine to medium Sand, trace Silt, trace fine Gravel, loose-dry to damp							
	X	8					2					
5												
					Dark Brown to Gray Brown Silty Clay, trace to little fine Sand, slightly porous, very stiff-damp to moist							
	X	20	4.25				10					
					Dark Brown Silty Clay, trace fine Sand, very stiff-damp to moist							
	X	25	4.5+				11					
10												
	X	27	4.5+				9					
15												
					Brown Silty fine Sand, little to some Clay, medium dense-damp to moist							
	X	19	3.5				9					
20												

Boring Terminated at 20'

TBL 14G160.GPJ SOCALGEO.GDT 3/24/15

SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

BORING NO. B-6

JOB NO.: 14G160
PROJECT: Prop. Moreno Valley Logistics Center
LOCATION: Moreno Valley, California

DRILLING DATE: 6/24/14
DRILLING METHOD: Hollow Stem Auger
LOGGED BY: Eric Torres

WATER DEPTH: Dry
CAVE DEPTH: 18 feet
READING TAKEN: At Completion

FIELD RESULTS						LABORATORY RESULTS						
DEPTH (FEET)	SAMPLE	BLOW COUNT	POCKET PEN. (TSF)	GRAPHIC LOG	DESCRIPTION SURFACE ELEVATION: --- MSL	DRY DENSITY (PCF)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PASSING #200 SIEVE (%)	UNCONFINED SHEAR (TSF)	COMMENTS
	X	26			ALLUVIUM: Brown Silty fine Sand to fine Sandy Silt, medium dense-dry to damp	111	3					
	X	32				102	3					
5	X	54			Brown Clayey Silt, trace fine Sand, trace calcareous veining, hard-dry to damp	102	6					
	X	55	1.5			106	5					
					Dark Brown Silty Clay, hard-damp							
10	X	78			Dark Brown Clayey fine Sand to fine Sandy Clay, little to some Silt, very dense-damp	111	5					
15	X	22	3.75		Dark Brown to Red Brown fine Sandy Clay, trace Silt, very stiff-damp to moist		10					
	X	40			Dark Brown fine to medium Sand, trace Silt, slightly cemented, medium dense-damp		6					
20					Brown Silty fine Sand, little Clay, meidum dense-damp to moist		10					

Boring Terminated at 20'

TBL 14G160.GPJ SOCALGEO.GDT 3/24/15

TEST BORING LOG **PLATE B-6**

SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

BORING NO. B-7

JOB NO.: 14G160
PROJECT: Prop. Moreno Valley Logistics Center
LOCATION: Moreno Valley, California
DRILLING DATE: 6/24/14
DRILLING METHOD: Hollow Stem Auger
LOGGED BY: Eric Torres
WATER DEPTH: Dry
CAVE DEPTH: 18 feet
READING TAKEN: At Completion

FIELD RESULTS						LABORATORY RESULTS						
DEPTH (FEET)	SAMPLE	BLOW COUNT	POCKET PEN. (TSF)	GRAPHIC LOG	DESCRIPTION SURFACE ELEVATION: --- MSL	DRY DENSITY (PCF)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PASSING #200 SIEVE (%)	UNCONFINED SHEAR (TSF)	COMMENTS
					ALLUVIUM: Brown Silty fine Sand, trace to little Clay, medium dense-dry to damp							
	X	22					4					
					Brown fine Sandy Silt, trace Clay, trace calcareous veining, medium dense-dry to damp							
5	X	21					6					
					Boring Terminated at 5'							

TBL 14G160.GPJ SOCALGEO.GDT 3/24/15

SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

BORING NO. B-8

JOB NO.: 14G160 DRILLING DATE: 6/25/14 WATER DEPTH: Dry
PROJECT: Prop. Moreno Valley Logistics Center DRILLING METHOD: Hollow Stem Auger CAVE DEPTH: 18 feet
LOCATION: Moreno Valley, California LOGGED BY: Eric Torres READING TAKEN: At Completion

FIELD RESULTS					DESCRIPTION	LABORATORY RESULTS						
DEPTH (FEET)	SAMPLE	BLOW COUNT	POCKET PEN. (TSF)	GRAPHIC LOG	SURFACE ELEVATION: --- MSL	DRY DENSITY (PCF)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PASSING #200 SIEVE (%)	UNCONFINED SHEAR (TSF)	COMMENTS
	X	62			ALLUVIUM: Brown Silty fine Sand, little Clay, very dense-dry to damp		2					
	X	35	4.5+		Gray Brown to Brown Clayey fine Sand to fine Sandy Clay, trace to little Silt, trace calcareous veining, very stiff to medium dense to dense-damp		6					
5	X	25	4.5+				6					
	X	28	2.0 4.5+		Dark Brown to Gray Brown Silty Clay, trace fine Sand, very stiff-damp to moist		7 11					
10					Gray Brown fine Sandy Clay, trace Silt, very stiff-moist							
	X	23	3.0				13					
15					Gray Brown Silty fine Sand, trace Silt, trace medium Sand, medium dense-damp to moist							
	X	23					8					
20					Boring Terminated at 20'							

TBL 14G160.GPJ SOCALGEO.GDT 3/24/15

SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

BORING NO. B-9

JOB NO.: 14G160
PROJECT: Prop. Moreno Valley Logistics Center
LOCATION: Moreno Valley, California

DRILLING DATE: 6/25/14
DRILLING METHOD: Hollow Stem Auger
LOGGED BY: Eric Torres

WATER DEPTH: Dry
CAVE DEPTH: 18 feet
READING TAKEN: At Completion

SURFACE ELEVATION: --- MSL

FIELD RESULTS						LABORATORY RESULTS						
DEPTH (FEET)	SAMPLE	BLOW COUNT	POCKET PEN. (TSF)	GRAPHIC LOG	DESCRIPTION	DRY DENSITY (PCF)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PASSING #200 SIEVE (%)	UNCONFINED SHEAR (TSF)	COMMENTS
					ALLUVIUM: Dark Brown Clayey fine Sand, little Silt, slightly porous, medium dense-dry to damp							
	X	31				114	2					
					Brown Silty fine Sand, trace to little Clay, medium dense-damp							
	X	43				110	3					
5					Brown fine to medium Sand, trace Silt, trace Clay, medium dense-damp							
	X	33				107	3					
					Brown Clayey fine Sand to fine Sandy Clay, little Silt, trace medium Sand, medium dense-damp							
	X	56	4.5+			118	6					
					Brown to Red Brown fine Sandy Clay, trace Silt, hard-damp to moist							
	X	49	4.5+			116	11					
10												
					Red Brown Clayey fine Sand, trace Silt, medium dense-moist							
	X	32					12					
15												

Boring Terminated at 15'

TBL 14G160.GPJ SOCALGEO.GDT 3/24/15

TEST BORING LOG

BORING NO. B-10

JOB NO.: 14G160
PROJECT: Prop. Moreno Valley Logistics Center
LOCATION: Moreno Valley, California
DRILLING DATE: 6/25/14
DRILLING METHOD: Hollow Stem Auger
LOGGED BY: Eric Torres
WATER DEPTH: Dry
CAVE DEPTH: 18 feet
READING TAKEN: At Completion

FIELD RESULTS						LABORATORY RESULTS						
DEPTH (FEET)	SAMPLE	BLOW COUNT	POCKET PEN. (TSF)	GRAPHIC LOG	DESCRIPTION SURFACE ELEVATION: --- MSL	DRY DENSITY (PCF)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PASSING #200 SIEVE (%)	UNCONFINED SHEAR (TSF)	COMMENTS
					ALLUVIUM: Dark Brown Clayey Silt, trace to little fine Sand, hard-dry to damp							
	X	31	4.5+				4					EI = 33 @ 0 to 5 feet
					Brown Silty fine Sand, little to some Silt, medium dense-damp to moist							
	X	20					10					
5												
	X	27					11					
					Brown Silty fine Sand, little Clay, dense-damp to moist							
	X	38					9					
10												
					Brown to Red Brown Silty Clay, trace to little fine Sand, trace calcareous veining, very stiff-moist							
	X	20	4.5+				13					
15												
					Brown fine Sandy Clay, trace to little Silt, very stiff-moist							
	X	19	3.0				14					
20												
					Dark Brown fine Sandy Clay, hard-moist							
	X	46	4.5+				13					
25												
					Boring Terminated at 25'							

TBL 14G160.GPJ SOCALGEO.GDT 3/24/15

TEST BORING LOG

BORING NO. B-11

SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

JOB NO.: 14G160
PROJECT: Prop. Moreno Valley Logistics Center
LOCATION: Moreno Valley, California

DRILLING DATE: 6/25/14
DRILLING METHOD: Hollow Stem Auger
LOGGED BY: Eric Torres

WATER DEPTH: Dry
CAVE DEPTH: 18 feet
READING TAKEN: At Completion

SURFACE ELEVATION: --- MSL

DEPTH (FEET)	SAMPLE	BLOW COUNT	POCKET PEN. (TSF)	GRAPHIC LOG	DESCRIPTION	DRY DENSITY (PCF)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PASSING #200 SIEVE (%)	UNCONFINED SHEAR (TSF)	COMMENTS
					ALLUVIUM: Brown Silty fine Sand, trace Clay, medium dense-dry to damp							
	X	31				110	4					
					Brown fine Sandy Clay, trace to little Silt, trace calcareous veining, very stiff to hard-damp							
	X	49	4.5+			114	9					
5					Brown Clayey Silt, trace fine Sand, very stiff-damp							
	X	33	4.5+			108	8					
	X	43	4.5+			116	8					
					Brown Silty fine Sand, little to some Clay, trace medium Sand, medium dense-damp							
	X	36				117	5					
10												
	X	31					6					
15					Brown to Gray Brown Silty fine to medium Sand, trace Silt, trace Clay, slightly cemented, dense-moist		14					
					Brown to Red Brown Clayey fine Sand, little Silt, medium dense-damp to moist							
	X	25					10					
					Brown Silty fine Sand, trace Clay, medium dense-damp to moist		9					
20												
					Dark Brown Clayey fine to medium Sand, trace Silt, medium dense-damp to moist							
	X	23					9					
25												
	X	26					8					
30												
					Boring Terminated at 30'							

SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

BORING NO. B-12

JOB NO.: 14G160
PROJECT: Prop. Moreno Valley Logistics Center
LOCATION: Moreno Valley, California

DRILLING DATE: 6/25/14
DRILLING METHOD: Hollow Stem Auger
LOGGED BY: Eric Torres

WATER DEPTH: Dry
CAVE DEPTH: 18 feet
READING TAKEN: At Completion

FIELD RESULTS						LABORATORY RESULTS						
DEPTH (FEET)	SAMPLE	BLOW COUNT	POCKET PEN. (TSF)	GRAPHIC LOG	DESCRIPTION SURFACE ELEVATION: --- MSL	DRY DENSITY (PCF)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PASSING #200 SIEVE (%)	UNCONFINED SHEAR (TSF)	COMMENTS
					ALLUVIUM: Dark Brown to Brown Silty fine Sand, little Clay, trace medium Sand, medium dense-dry to damp							
		21				102	4					
		12				107	5					
5		25				115	7					
					Dark Brown fine Sandy Clay, trace Silt, slightly cemented, very stiff to hard-damp to moist							
		55	4.5+			118	13					
		24	3.75			107	25					
10					Brown Silty Clay, trace fine Sand, very stiff-moist to very moist							
					Brown Clayey fine Sand, little to some Silt, medium dense-moist							
		15					12					
					Brown Silty fine Sand, trace medium Sand, trace Clay, medium dense-damp to moist		8					
15												
					Brown fine Sandy Clay, trace Silt, very stiff-damp to moist							
		18	3.5				12					
20												
					Brown fine to medium Sand, little to some Clay, trace Silt, medium dense-moist							
		25					11					
25			4.5+		Gray Brown Silty Clay, trace fine Sand, very stiff-moist		15					
					Boring Terminated at 25'							

SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

BORING NO. B-13

JOB NO.: 14G160
PROJECT: Prop. Moreno Valley Logistics Center
LOCATION: Moreno Valley, California

DRILLING DATE: 6/25/14
DRILLING METHOD: Hollow Stem Auger
LOGGED BY: Eric Torres

WATER DEPTH: Dry
CAVE DEPTH: 18 feet
READING TAKEN: At Completion

FIELD RESULTS						LABORATORY RESULTS						
DEPTH (FEET)	SAMPLE	BLOW COUNT	POCKET PEN. (TSF)	GRAPHIC LOG	DESCRIPTION SURFACE ELEVATION: --- MSL	DRY DENSITY (PCF)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PASSING #200 SIEVE (%)	UNCONFINED SHEAR (TSF)	COMMENTS
					ALLUVIUM: Dark Brown Silty fine Sand, medium dense-dry to damp							
	X	26					3					EI = 0 @ 0 to 5 feet
					Brown Clayey Silt, trace to little Clay, very stiff-damp to moist							
	X	24	2.5				8					
5					Dark Brown Silty Clay, trace fine Sand, very stiff-damp to moist							
	X	16	2.5				10					
	X	14	3.0				12					
10												
					Dark Brown Clayey fine to medium Sand, trace Silt, stiff-damp to moist							
	X	21					9					
15					Boring Terminated at 15'							

TBL 14G160.GPJ SOCALGEO.GDT 3/24/15

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A California Corporation

BORING NO. B-14

JOB NO.: 14G160 | DRILLING DATE: 6/25/14 | WATER DEPTH: Dry
PROJECT: Prop. Moreno Valley Logistics Center | DRILLING METHOD: Hollow Stem Auger | CAVE DEPTH: 18 feet
LOCATION: Moreno Valley, California | LOGGED BY: Eric Torres | READING TAKEN: At Completion

FIELD RESULTS						LABORATORY RESULTS						
DEPTH (FEET)	SAMPLE	BLOW COUNT	POCKET PEN. (TSF)	GRAPHIC LOG	DESCRIPTION SURFACE ELEVATION: --- MSL	DRY DENSITY (PCF)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PASSING #200 SIEVE (%)	UNCONFINED SHEAR (TSF)	COMMENTS
					<u>ALLUVIUM:</u> Dark Brown Silty fine Sand, trace to little Clay, loose to medium dense-dry to damp							
		19				107	3					
		9				103	4					
5		20			Brown Clayey fine Sand, trace Silt, medium dense-moist	106	16					
		46	4.5+		Brown fine Sandy Clay, little Silt, very stiff-damp to moist	119	8					
10		16			Gray Brown Silty fine Sand, trace Clay, medium dense-damp to moist	110	8					
					Brown to Gray Brown fine Sandy Silt, trace Clay, stiff to very stiff-moist							
15		15					12					
		28					16					
20					Boring Terminated at 20'							

TBL 14G160.GPJ SOCALGEO.GDT 3/24/15

SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

BORING NO. B-15

JOB NO.: 14G160 | DRILLING DATE: 6/25/14 | WATER DEPTH: Dry
PROJECT: Prop. Moreno Valley Logistics Center | DRILLING METHOD: Hollow Stem Auger | CAVE DEPTH: 18 feet
LOCATION: Moreno Valley, California | LOGGED BY: Eric Torres | READING TAKEN: At Completion

FIELD RESULTS					DESCRIPTION	LABORATORY RESULTS						
DEPTH (FEET)	SAMPLE	BLOW COUNT	POCKET PEN. (TSF)	GRAPHIC LOG	SURFACE ELEVATION: --- MSL	DRY DENSITY (PCF)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PASSING #200 SIEVE (%)	UNCONFINED SHEAR (TSF)	COMMENTS
	X	30	4.5+		ALLUVIUM: Brown fine Sandy Clay, little to some Silt, trace calcareous veining, very stiff to hard-dry to damp		6					
5	X	24			Light Brown fine Sandy Silt, medium dense-damp to moist		8					
					Boring Terminated at 5'							

TBL 14G160.GPJ SOCALGEO.GDT 3/24/15

TEST BORING LOG

SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

BORING NO. B-16

JOB NO.: 14G160
PROJECT: Prop. Moreno Valley Logistics Center
LOCATION: Moreno Valley, California

DRILLING DATE: 3/12/15
DRILLING METHOD: Hollow Stem Auger
LOGGED BY: Daryl Kas

WATER DEPTH: Dry
CAVE DEPTH: 12 feet
READING TAKEN: At Completion

SURFACE ELEVATION: --- MSL

FIELD RESULTS						LABORATORY RESULTS						
DEPTH (FEET)	SAMPLE	BLOW COUNT	POCKET PEN. (TSF)	GRAPHIC LOG	DESCRIPTION	DRY DENSITY (PCF)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PASSING #200 SIEVE (%)	UNCONFINED SHEAR (TSF)	COMMENTS
	X	23			ALLUVIUM: Brown Silty fine Sand, trace Clay, trace medium Sand, medium dense-dry to damp	102	5					EI = 1 @ 0 to 5'
	X	28			Brown Silty fine to medium Sand, trace coarse Sand, trace Clay, medium dense-damp	115	5					
5	X	19			Brown fine to coarse Sand, some Silt, medium dense-damp	109	3					
	X	41	4.5+		Dark Brown Silty Clay, trace fine Sand, trace calcareous veining, very stiff-damp to moist	122	12					
10	X	80			Orange Brown Silty fine Sand, slightly cemnented, very dense-damp to moist	111	8					
15	X	18			Dark Brown Clayey fine Sand, trace medium Sand, some Silt, medium dense-damp to moist		8					
	X	25			Light Brown fine to coarse Sand, trace Silt, medium dense-damp		3					
20			3.5		Brown fine Sandy Clay, some Silt, very stiff-damp to moist		12					
25	X	21	4.0				13					

Boring Terminated at 25'

TBL 14G160.GPJ SOCALGEO.GDT 3/24/15

TEST BORING LOG

SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

BORING NO. B-17

JOB NO.: 14G160
PROJECT: Prop. Moreno Valley Logistics Center
LOCATION: Moreno Valley, California

DRILLING DATE: 3/12/15
DRILLING METHOD: Hollow Stem Auger
LOGGED BY: Daryl Kas

WATER DEPTH: Dry
CAVE DEPTH: 12 feet
READING TAKEN: At Completion

SURFACE ELEVATION: --- MSL

DEPTH (FEET)	SAMPLE	BLOW COUNT	POCKET PEN. (TSF)	GRAPHIC LOG	DESCRIPTION	DRY DENSITY (PCF)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PASSING #200 SIEVE (%)	UNCONFINED SHEAR (TSF)	COMMENTS
					ALLUVIUM: Brown Silty fine to medium Sand, trace coarse Sand, trace Clay, medium dense-damp							
	X	25				107	5					
	X	32				118	6					
5					Light Brown Silty fine Sand, trace to little medium Sand, trace Clay, medium dense-damp							
	X	33				116	5					
					Light Gray Brown fine Sandy Silt, trace Clay, slightly cemented, dense-damp to moist							
	X	60				116	7					
					Light Orange Brown Silty fine Sand, little calcareous veining, slightly cemented, very dense-damp to moist							
10	X	74/9"				108	10					
					Brown Silty fine Sand, little medium Sand, little calcareous veining, dense-damp to moist							
	X	32					8					
15												
					Brown Clayey fine Sand, medium dense-moist							
	X	16					13					
20												

Boring Terminated at 20'

TBL 14G160.GPJ SOCALGEO.GDT 3/24/15

BORING NO. B-18

JOB NO.: 14G160	DRILLING DATE: 3/12/15	WATER DEPTH: Dry
PROJECT: Prop. Moreno Valley Logistics Center	DRILLING METHOD: Hollow Stem Auger	CAVE DEPTH: 3 feet
LOCATION: Moreno Valley, California	LOGGED BY: Daryl Kas	READING TAKEN: At Completion

FIELD RESULTS						LABORATORY RESULTS						
DEPTH (FEET)	SAMPLE	BLOW COUNT	POCKET PEN. (TSF)	GRAPHIC LOG	DESCRIPTION SURFACE ELEVATION: --- MSL	DRY DENSITY (PCF)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PASSING #200 SIEVE (%)	UNCONFINED SHEAR (TSF)	COMMENTS
					ALLUVIUM: Gray Brown Clayey fine Sand, trace medium Sand, medium dense-damp							
	X	22					5					
	X	22			@ 3½ feet, moderately cemented		8					
5												
					Boring Terminated at 5'							

TBL 14G160.GPJ SOCALGEO.GDT 3/24/15

SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

BORING NO. B-19

JOB NO.: 14G160	DRILLING DATE: 3/12/15	WATER DEPTH: 27 feet
PROJECT: Prop. Moreno Valley Logistics Center	DRILLING METHOD: Hollow Stem Auger	CAVE DEPTH: 12 feet
LOCATION: Moreno Valley, California	LOGGED BY: Daryl Kas	READING TAKEN: At Completion

FIELD RESULTS						LABORATORY RESULTS						
DEPTH (FEET)	SAMPLE	BLOW COUNT	POCKET PEN. (TSF)	GRAPHIC LOG	DESCRIPTION SURFACE ELEVATION: --- MSL	DRY DENSITY (PCF)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PASSING #200 SIEVE (%)	UNCONFINED SHEAR (TSF)	COMMENTS
		19			ALLUVIUM: Gray Brown Silty fine Sand to fine Sandy Silt, trace coarse Sand, slightly porous, medium dense-dry to damp	105	2					
		21			Brown Silty fine to medium Sand, medium dense-dry to damp	108	2					
5		39			Gray Brown Silty fine Sand, trace Clay, little medium to coarse Sand, moderately cemented, medium dense-dry to damp	118	4					
		30			Gray Brown Clayey fine Sand, trace Silt, trace to little medium Sand, medium dense-damp	121	6					
10		28			Brown Silty fine Sand, some Clay, trace fine Sand, medium dense-moist	118	11					
15		30			Red Brown Silty fine Sand, medium dense to dense-moist		9					
20		11			Dark Gray Brown fine Sandy Silt, some Clay, medium dense-moist to very moist		21					
25		13	2.0		Gray Brown Clayey Silt, trace fine Sand, stiff-moist to very moist		19					
					@ 27 feet, Perched Groundwater encountered during drilling							
30		13	2.75				18					
					Boring Terminated at 30'							

TBL 14G160.GPJ SOCALGEO.GDT 3/24/15

BORING NO. B-20

SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

JOB NO.: 14G160
PROJECT: Prop. Moreno Valley Logistics Center
LOCATION: Moreno Valley, California
DRILLING DATE: 3/12/15
DRILLING METHOD: Hollow Stem Auger
LOGGED BY: Daryl Kas
WATER DEPTH: Dry
CAVE DEPTH: 12 feet
READING TAKEN: At Completion

FIELD RESULTS					DESCRIPTION	LABORATORY RESULTS						
DEPTH (FEET)	SAMPLE	BLOW COUNT	POCKET PEN. (TSF)	GRAPHIC LOG	SURFACE ELEVATION: --- MSL	DRY DENSITY (PCF)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PASSING #200 SIEVE (%)	UNCONFINED SHEAR (TSF)	COMMENTS
	X	12			ALLUVIUM: Gray Brown Silty fine Sand, medium dense-damp		5					
5	X	24			Gray Brown Clayey fine Sand, medium dense-damp		4					
	X	23			Brown Silty fine to coarse Sand, medium dense-damp		4					
10	X	25	4.5+		Gray Brown Silty Clay, trace to little fine Sand, moderately cemented, very stiff-damp to moist		13					
15	X	18			Light Brown Silty fine to coarse Sand, medium dense-damp		5					
20	X	42			Gray Brown Clayey fine Sand, dense-damp		7					
					Boring Terminated at 20'							

TBL 14G160.GPJ SOCALGEO.GDT 3/24/15

TEST BORING LOG **PLATE B-20**

SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

BORING NO. B-21

JOB NO.: 14G160
PROJECT: Prop. Moreno Valley Logistics Center
LOCATION: Moreno Valley, California

DRILLING DATE: 3/12/15
DRILLING METHOD: Hollow Stem Auger
LOGGED BY: Daryl Kas

WATER DEPTH: Dry
CAVE DEPTH: 15 feet
READING TAKEN: At Completion

SURFACE ELEVATION: --- MSL

DEPTH (FEET)	BLOW COUNT	POCKET PEN. (TSF)	DESCRIPTION	DRY DENSITY (PCF)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PASSING #200 SIEVE (%)	UNCONFINED SHEAR (TSF)	COMMENTS
	22		ALLUVIUM: Dark Gray Brown Silty fine Sand, porous, medium dense-dry to damp	102	4					EI = 0 @ 0 to 5'
			Light Brown Silty fine Sand, trace coarse Sand, medium dense-damp							
	25		Brown Silty fine to coarse Sand, medium dense-damp	107	3					
5	38			120	2					
			Gray Brown Clayey fine Sand, medium dense-damp							
	59		Light Gray Brown Silty fine to medium Sand, trace Clay, moderately cemented, dense-damp	115	5					
10	61		Light Brown Silty fine Sand, slightly cemented, trace calcareous nodules, some calcareous veining, dense-moist	112	11					
15	21	1.5	Brown Clayey Silt, some fine Sand, slightly cemented, trace calcareous veining, very stiff-damp to moist		8					
	10	1.5	Light Gray Brown Clayey Silt, trace fine Sand, stiff-moist to very moist		18					
20	33		Gray Brown Clayey fine Sand, trace medium Sand, medium dense-very moist	117	14					
	19				15					
25			Boring Terminated at 25'							

SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

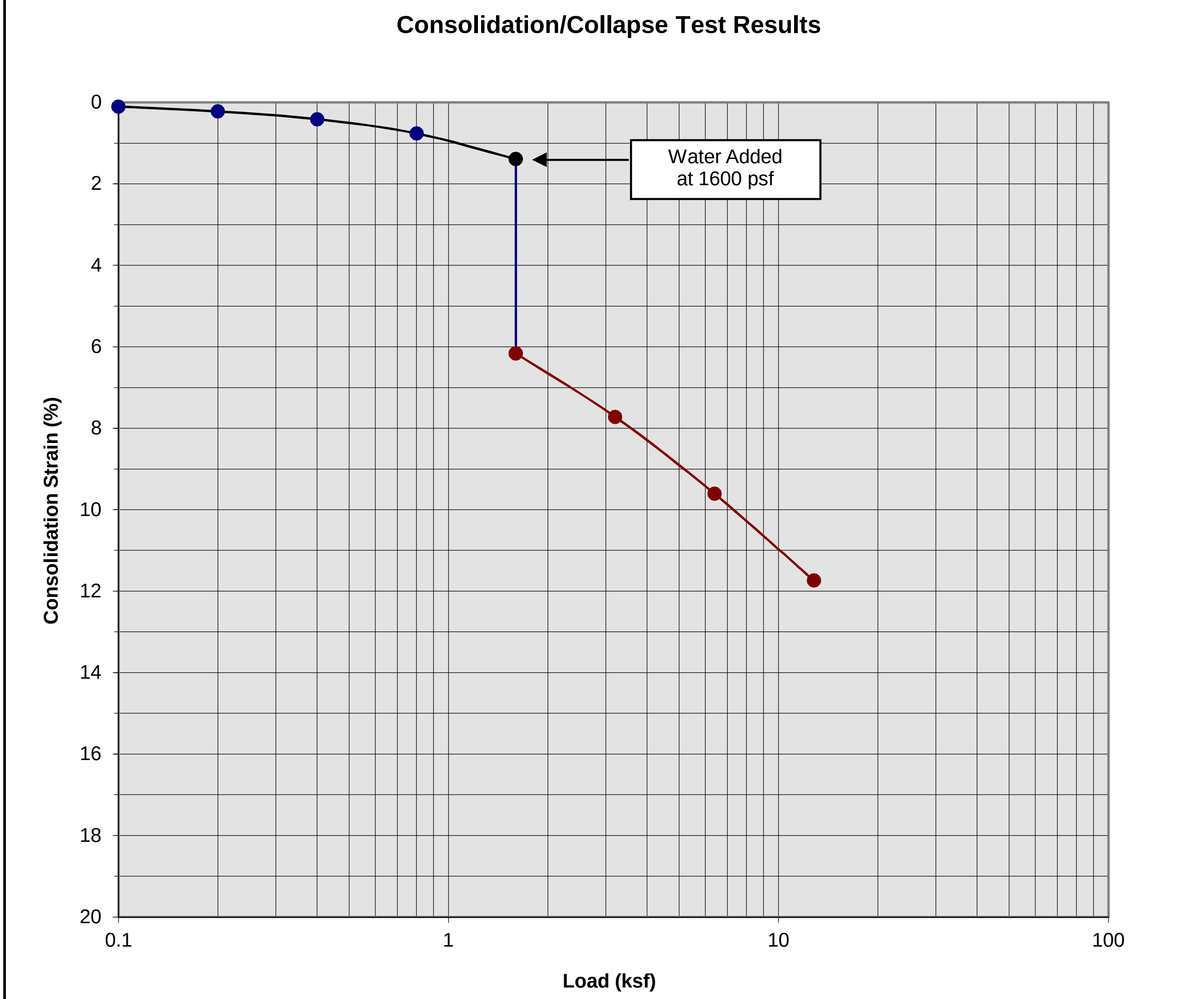
BORING NO. B-22

JOB NO.: 14G160 | DRILLING DATE: 3/12/15 | WATER DEPTH: Dry
PROJECT: Prop. Moreno Valley Logistics Center | DRILLING METHOD: Hollow Stem Auger | CAVE DEPTH: 3 feet
LOCATION: Moreno Valley, California | LOGGED BY: Daryl Kas | READING TAKEN: At Completion

FIELD RESULTS						LABORATORY RESULTS						
DEPTH (FEET)	SAMPLE	BLOW COUNT	POCKET PEN. (TSF)	GRAPHIC LOG	DESCRIPTION SURFACE ELEVATION: --- MSL	DRY DENSITY (PCF)	MOISTURE CONTENT (%)	LIQUID LIMIT	PLASTIC LIMIT	PASSING #200 SIEVE (%)	UNCONFINED SHEAR (TSF)	COMMENTS
	X	22			ALLUVIUM: Light Brown Silty fine Sand, medium dense-dry to damp		3					
5	X	39			Gray Brown Clayey fine Sand, trace medium Sand, dense-damp		4					
					Boring Terminated at 5'							

TBL 14G160.GPJ SOCALGEO.GDT 3/24/15

APPENDIX C



Classification: Brown Silty fine Sand, trace medium Sand

Boring Number:	B-2
Sample Number:	---
Depth (ft)	1 to 2
Specimen Diameter (in)	2.4
Specimen Thickness (in)	1.0

Initial Moisture Content (%)	2
Final Moisture Content (%)	14
Initial Dry Density (pcf)	105.2
Final Dry Density (pcf)	119.9
Percent Collapse (%)	4.77

Prop. Moreno Valley Logistics Center
Moreno Valley, California
Project No. 14G160
PLATE C- 1

SoCalGeo
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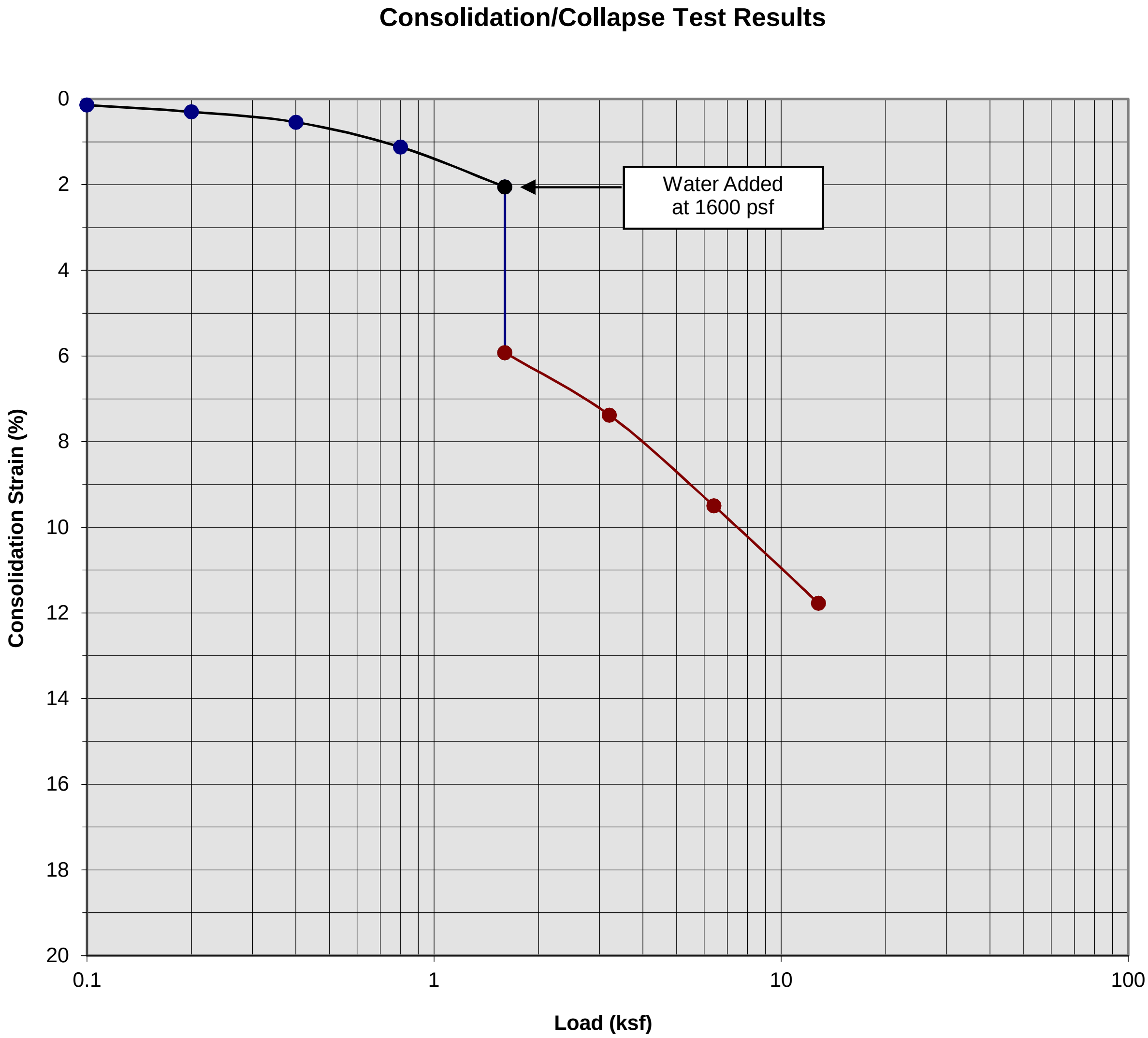


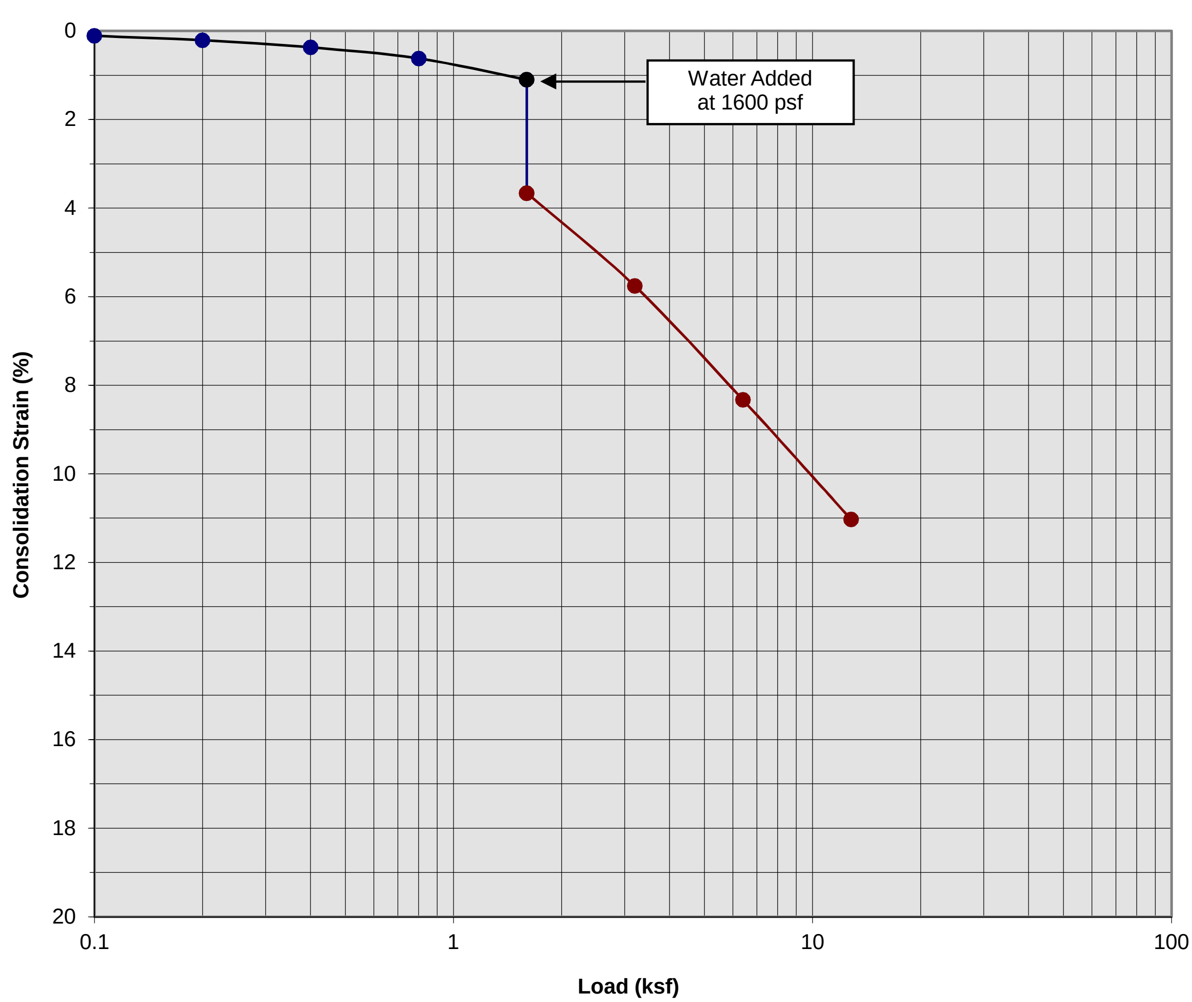
Classification: Brown Silty fine to medium Sand

Boring Number:	B-2	Initial Moisture Content (%)	3
Sample Number:	---	Final Moisture Content (%)	12
Depth (ft)	3 to 4	Initial Dry Density (pcf)	108.3
Specimen Diameter (in)	2.4	Final Dry Density (pcf)	122.7
Specimen Thickness (in)	1.0	Percent Collapse (%)	3.87

Prop. Moreno Valley Logistics Center
Moreno Valley, California
Project No. 14G160
PLATE C- 2

SoCalGeo
SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

Consolidation/Collapse Test Results

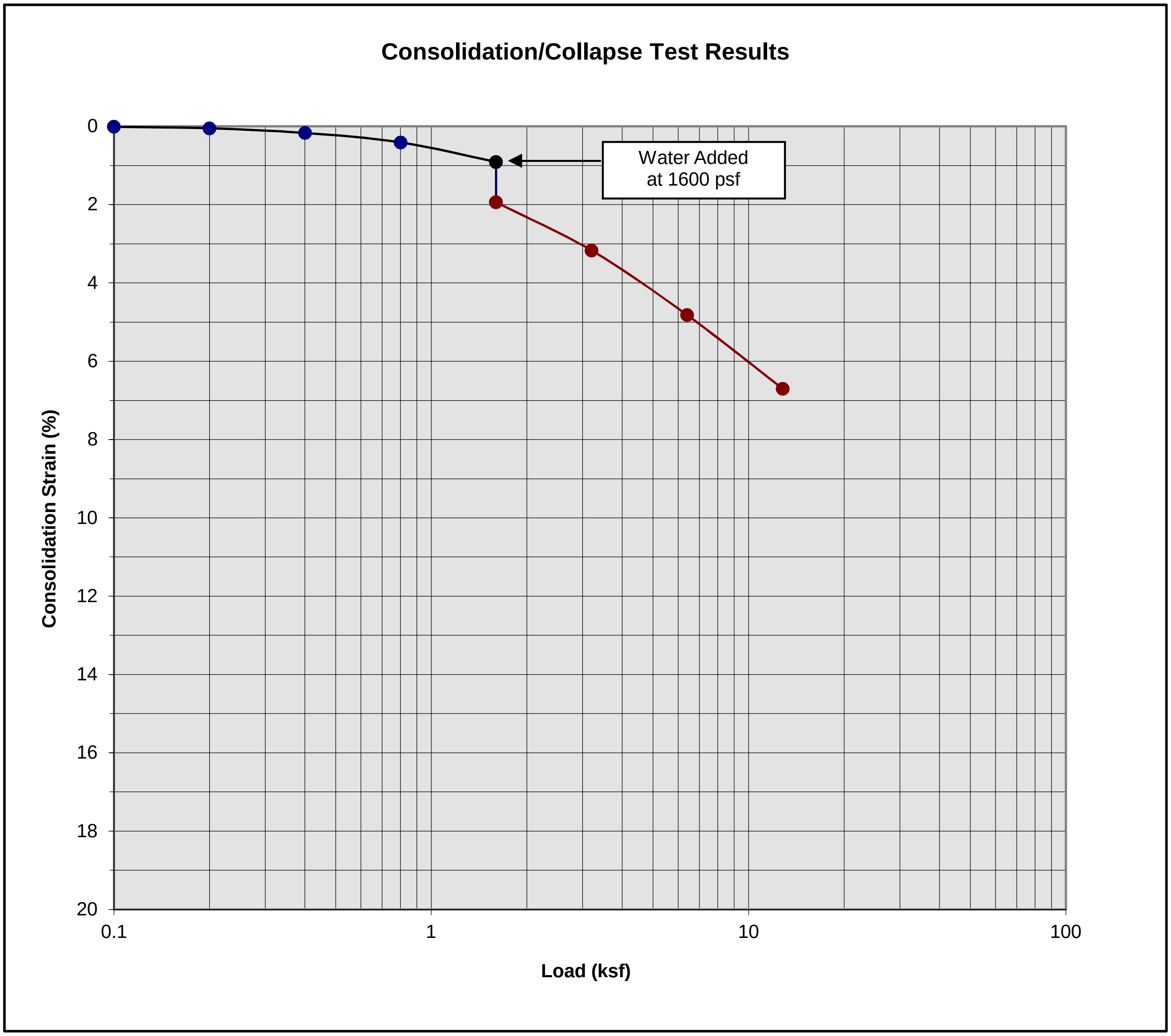


Classification: Brown Clayey fine Sand, little to some Silt, trace medium Sand

Boring Number:	B-2	Initial Moisture Content (%)	3
Sample Number:	---	Final Moisture Content (%)	13
Depth (ft)	5 to 6	Initial Dry Density (pcf)	123.3
Specimen Diameter (in)	2.4	Final Dry Density (pcf)	138.1
Specimen Thickness (in)	1.0	Percent Collapse (%)	2.56

Prop. Moreno Valley Logistics Center
Moreno Valley, California
Project No. 14G160
PLATE C- 3

SoCalGeo SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation



Classification: Dark Brown fine Sandy Clay, trace to little Silt

Boring Number:	B-2	Initial Moisture Content (%)	10
Sample Number:	---	Final Moisture Content (%)	13
Depth (ft)	7 to 8	Initial Dry Density (pcf)	122.5
Specimen Diameter (in)	2.4	Final Dry Density (pcf)	130.9
Specimen Thickness (in)	1.0	Percent Collapse (%)	1.03

Prop. Moreno Valley Logistics Center
Moreno Valley, California
Project No. 14G160
PLATE C- 4

SoCalGeo
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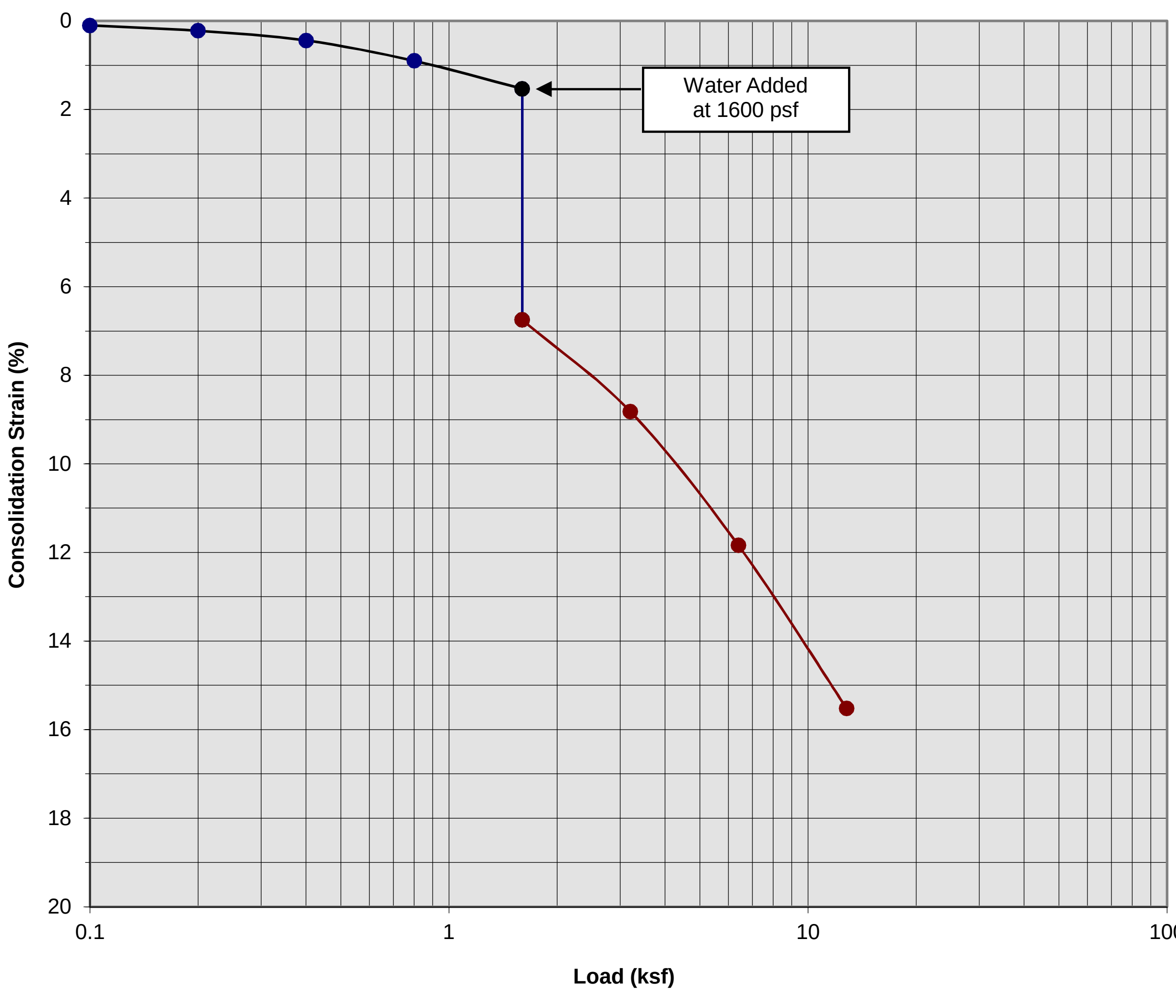
Consolidation/Collapse Test Results



Classification: Brown fine Sandy Clay, trace to little Silt

Boring Number:	B-11	Initial Moisture Content (%)	4
Sample Number:	---	Final Moisture Content (%)	13
Depth (ft)	1 to 2	Initial Dry Density (pcf)	109.6
Specimen Diameter (in)	2.4	Final Dry Density (pcf)	129.1
Specimen Thickness (in)	1.0	Percent Collapse (%)	5.22

Prop. Moreno Valley Logistics Center
Moreno Valley, California
Project No. 14G160
PLATE C- 5

SoCalGeo SOUTHERN CALIFORNIA GEOTECHNICAL
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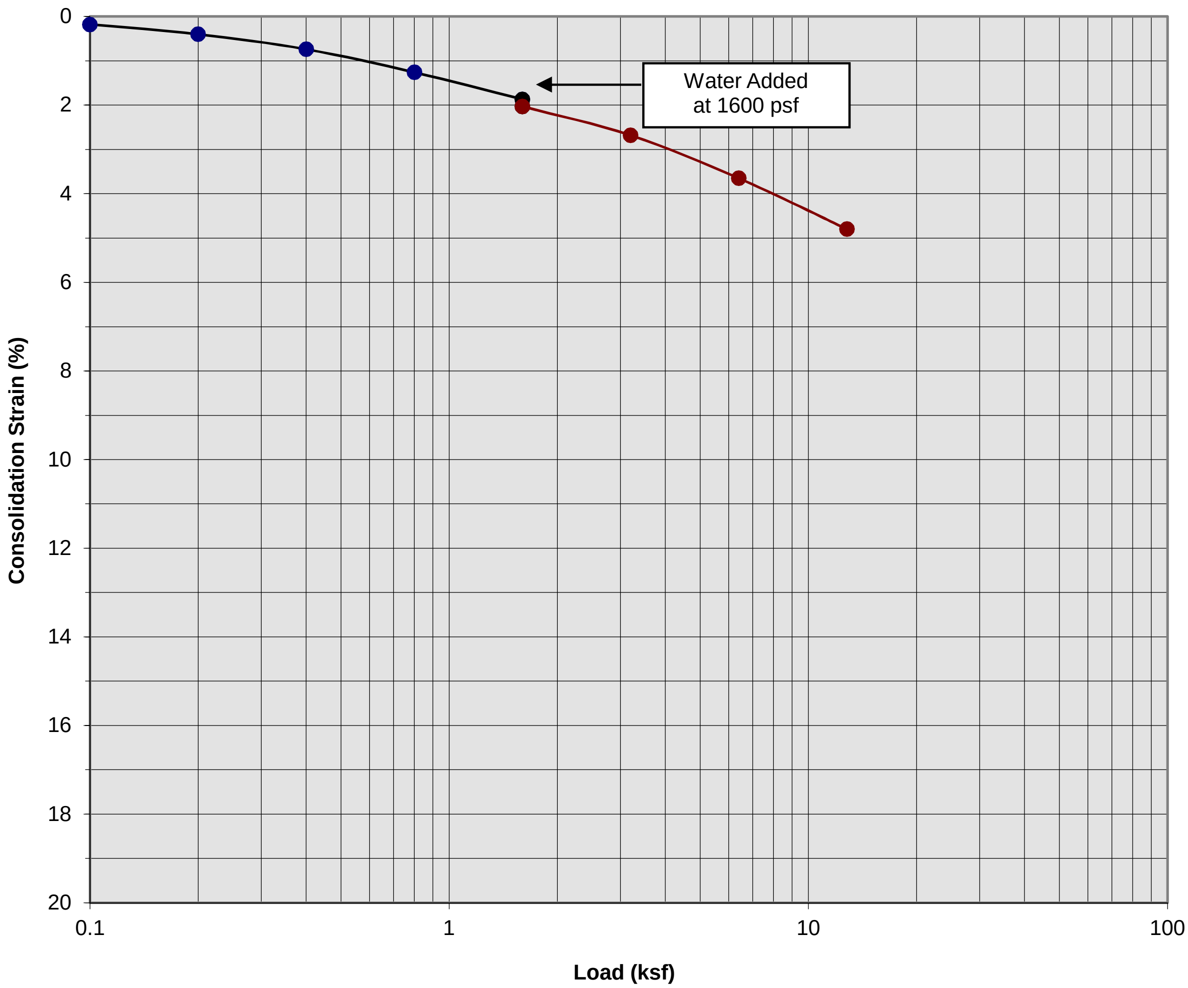
Consolidation/Collapse Test Results



Classification: Brown fine Sandy Clay, trace to little Silt

Boring Number:	B-11	Initial Moisture Content (%)	9
Sample Number:	---	Final Moisture Content (%)	16
Depth (ft)	3 to 4	Initial Dry Density (pcf)	114.3
Specimen Diameter (in)	2.4	Final Dry Density (pcf)	120.7
Specimen Thickness (in)	1.0	Percent Collapse (%)	0.16

Prop. Moreno Valley Logistics Center
Moreno Valley, California
Project No. 14G160
PLATE C- 6

SoCalGeo
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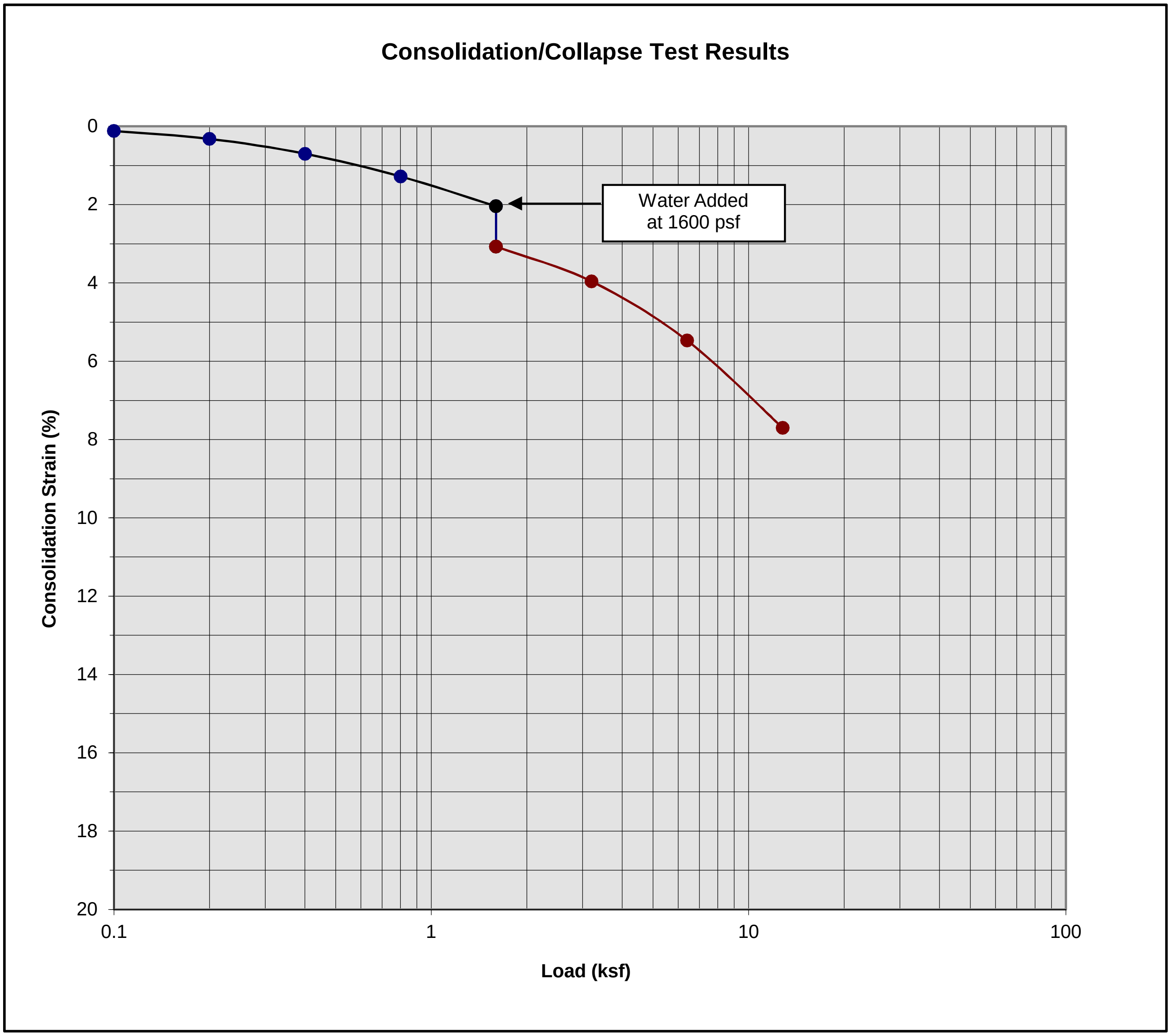

Classification: Brown Clayey Silt, trace fine Sand

Boring Number:	B-11	Initial Moisture Content (%)	8
Sample Number:	---	Final Moisture Content (%)	18
Depth (ft)	5 to 6	Initial Dry Density (pcf)	108.0
Specimen Diameter (in)	2.4	Final Dry Density (pcf)	117.6
Specimen Thickness (in)	1.0	Percent Collapse (%)	1.03

Prop. Moreno Valley Logistics Center
Moreno Valley, California
Project No. 14G160
PLATE C- 7

SoCalGeo SOUTHERN CALIFORNIA GEOTECHNICAL
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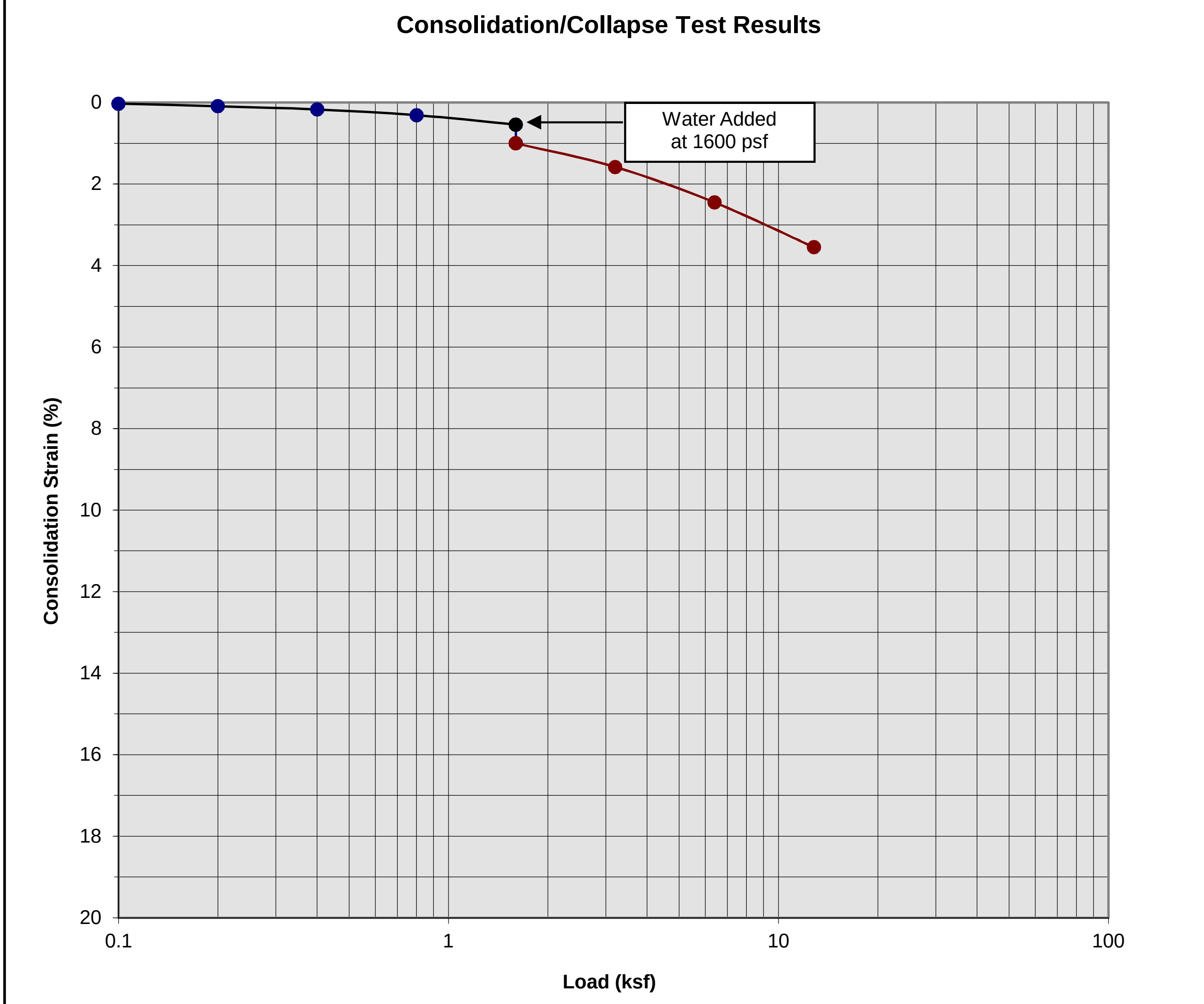

Classification: Brown Clayey Silt, trace fine Sand

Boring Number:	B-11	Initial Moisture Content (%)	8
Sample Number:	---	Final Moisture Content (%)	13
Depth (ft)	7 to 8	Initial Dry Density (pcf)	115.9
Specimen Diameter (in)	2.4	Final Dry Density (pcf)	120.6
Specimen Thickness (in)	1.0	Percent Collapse (%)	0.46

Prop. Moreno Valley Logistics Center
Moreno Valley, California
Project No. 14G160
PLATE C- 8

SoCalGeo SOUTHERN CALIFORNIA GEOTECHNICAL
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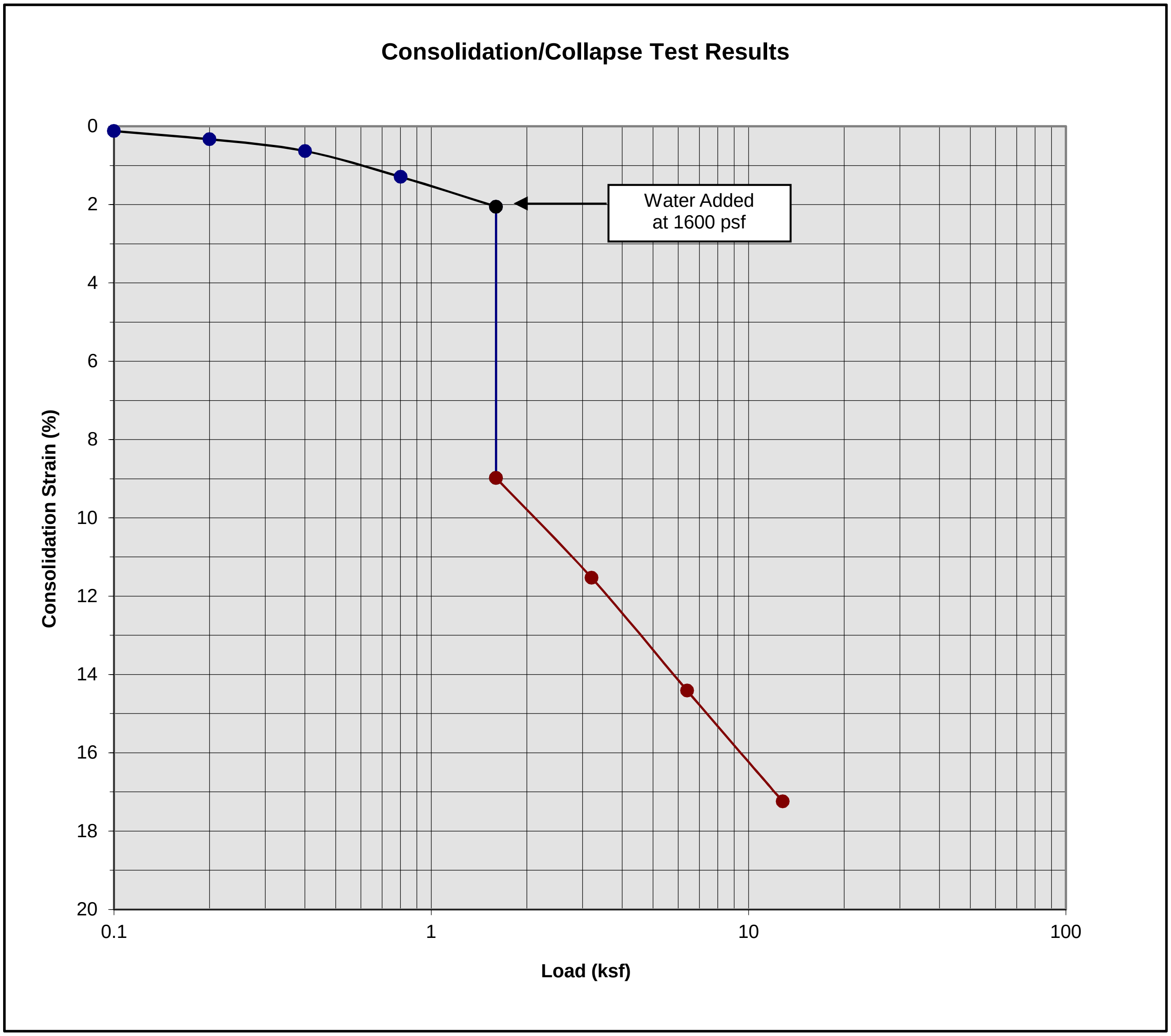

Classification: Dark Brown Silty fine Sand, trace to little Clay

Boring Number:	B-14	Initial Moisture Content (%)	3
Sample Number:	---	Final Moisture Content (%)	15
Depth (ft)	1 to 2	Initial Dry Density (pcf)	107.0
Specimen Diameter (in)	2.4	Final Dry Density (pcf)	129.8
Specimen Thickness (in)	1.0	Percent Collapse (%)	6.93

Prop. Moreno Valley Logistics Center
Moreno Valley, California
Project No. 14G160
PLATE C- 9

SoCalGeo SOUTHERN CALIFORNIA GEOTECHNICAL
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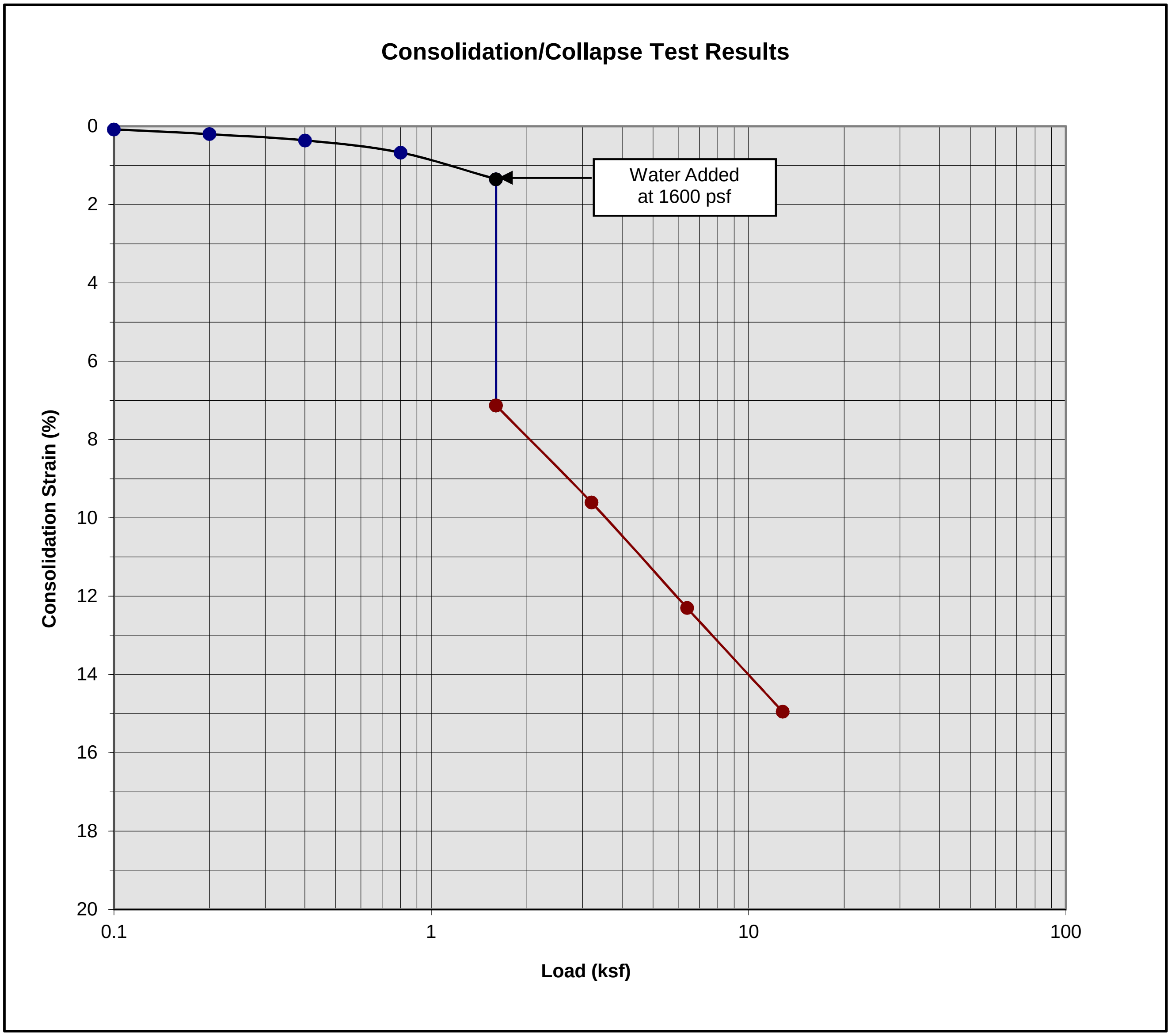

Classification: Dark Brown Silty fine Sand, trace to little Clay

Boring Number:	B-14	Initial Moisture Content (%)	3
Sample Number:	---	Final Moisture Content (%)	16
Depth (ft)	3 to 4	Initial Dry Density (pcf)	103.1
Specimen Diameter (in)	2.4	Final Dry Density (pcf)	120.8
Specimen Thickness (in)	1.0	Percent Collapse (%)	5.78

Prop. Moreno Valley Logistics Center
Moreno Valley, California
Project No. 14G160
PLATE C- 10

SoCalGeo SOUTHERN CALIFORNIA GEOTECHNICAL
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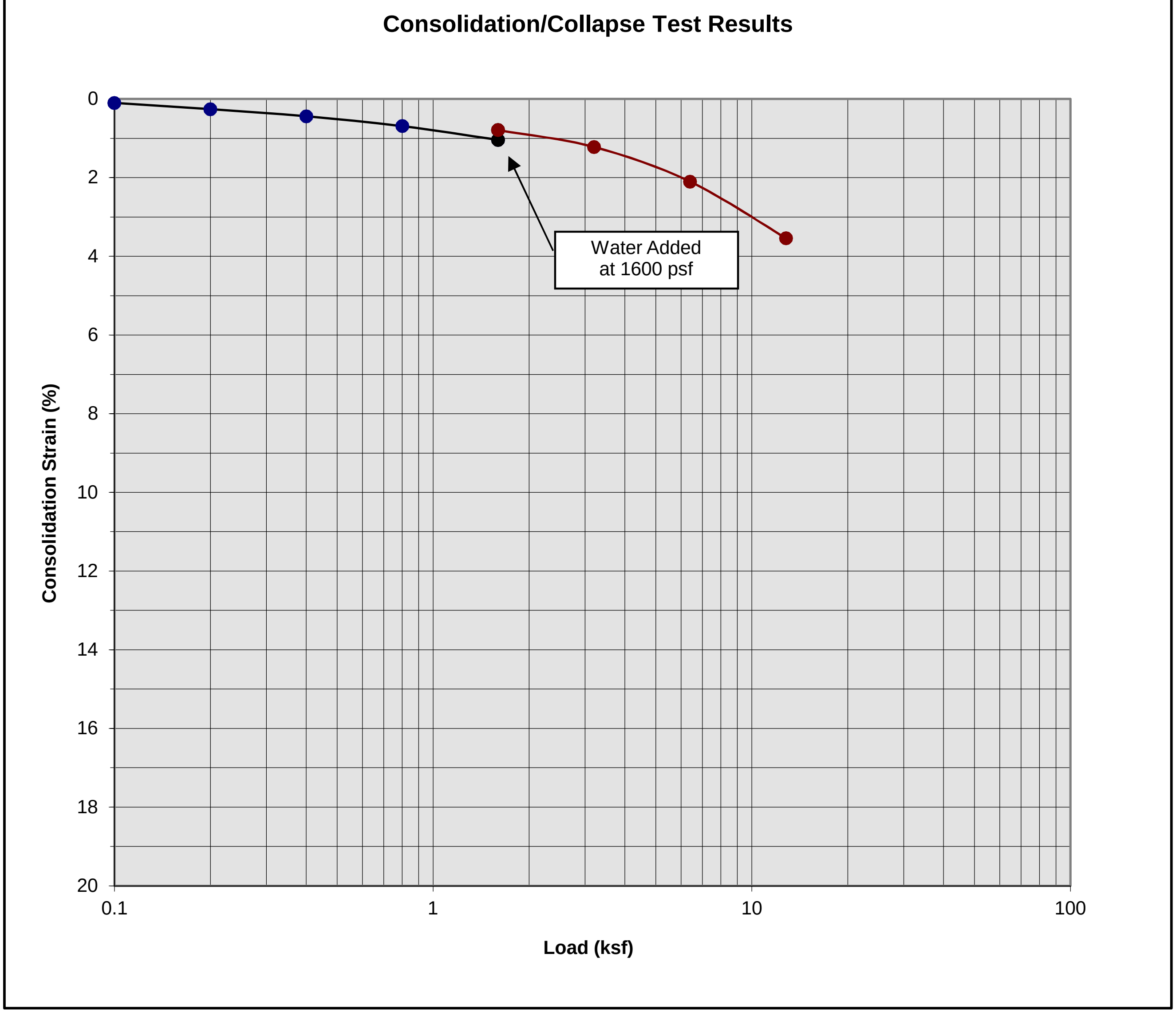

Classification: Dark Brown Silty fine Sand, trace to little Clay

Boring Number:	B-14	Initial Moisture Content (%)	16
Sample Number:	---	Final Moisture Content (%)	21
Depth (ft)	5 to 6	Initial Dry Density (pcf)	106.3
Specimen Diameter (in)	2.4	Final Dry Density (pcf)	110.4
Specimen Thickness (in)	1.0	Percent Collapse (%)	-0.25

Prop. Moreno Valley Logistics Center
Moreno Valley, California
Project No. 14G160
PLATE C- 11

SoCalGeo SOUTHERN CALIFORNIA GEOTECHNICAL
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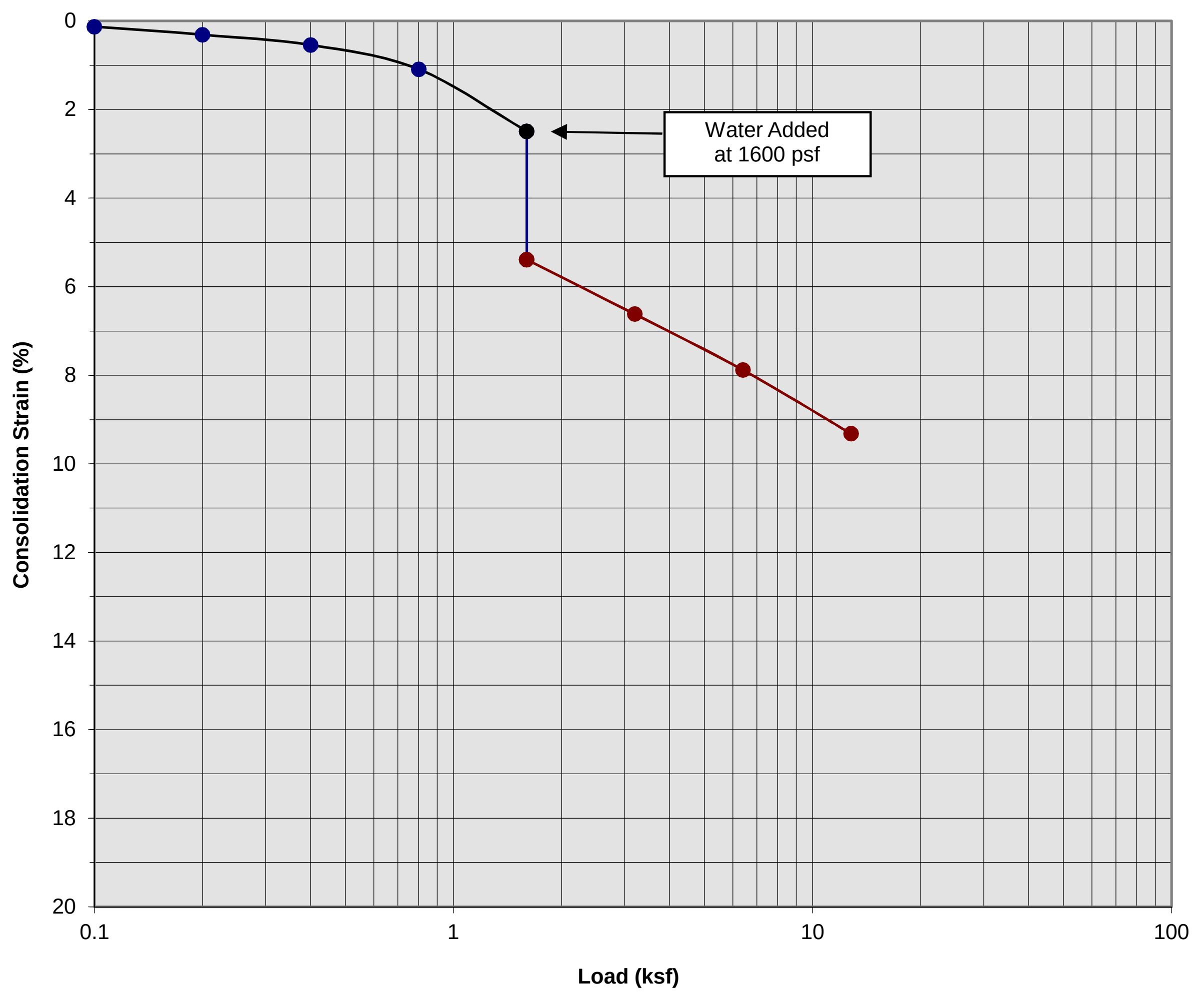
Consolidation/Collapse Test Results



Classification: Brown fine Sandy Clay, little Silt

Boring Number:	B-14	Initial Moisture Content (%)	8
Sample Number:	---	Final Moisture Content (%)	14
Depth (ft)	7 to 8	Initial Dry Density (pcf)	118.9
Specimen Diameter (in)	2.4	Final Dry Density (pcf)	131.6
Specimen Thickness (in)	1.0	Percent Collapse (%)	2.90

Prop. Moreno Valley Logistics Center
Moreno Valley, California
Project No. 14G160
PLATE C- 12

SoCalGeo
SOUTHERN CALIFORNIA GEOTECHNICAL
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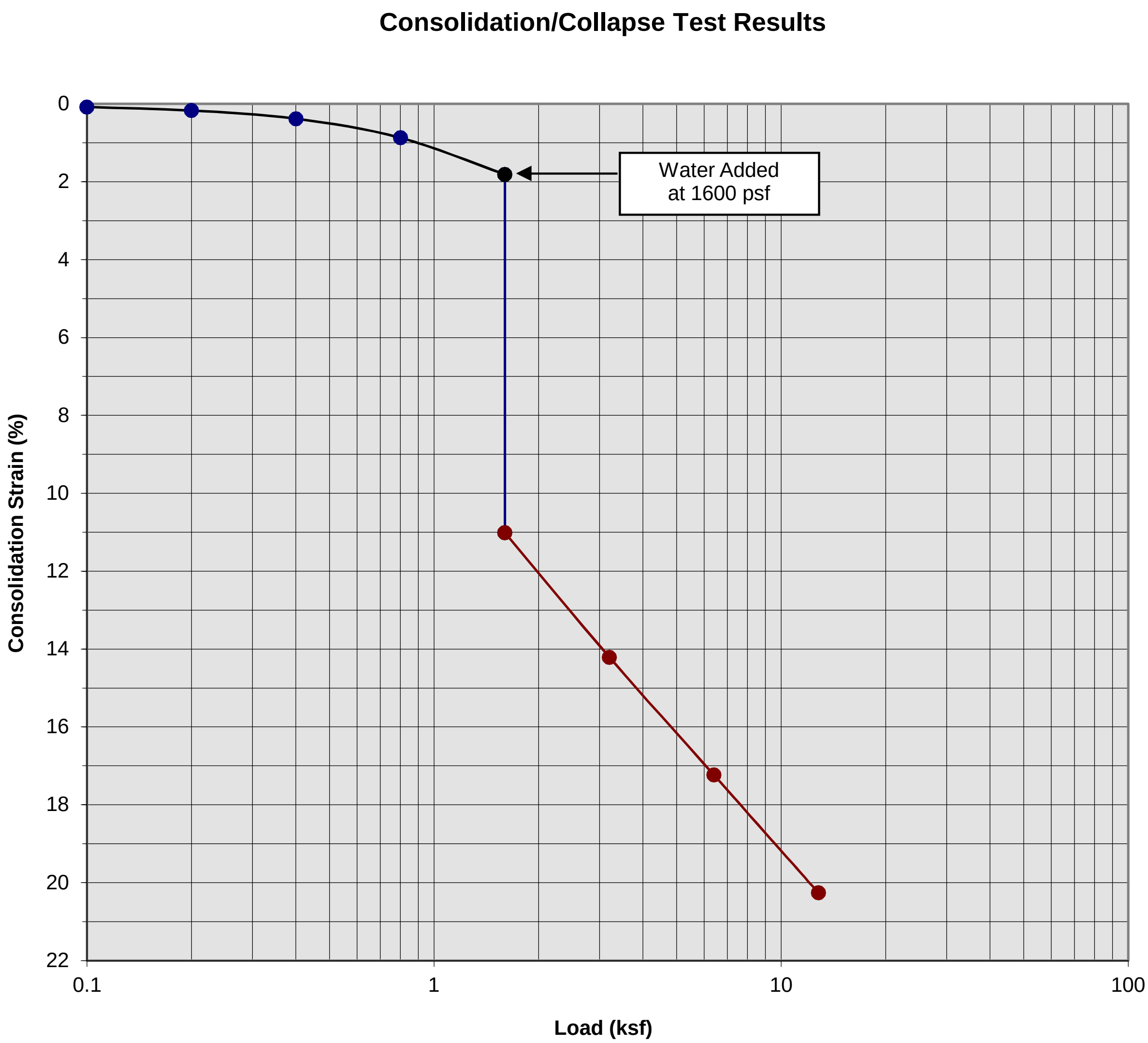

Classification: Brown Silty fine Sand, some Clay, trace medium Sand

Boring Number:	B-16	Initial Moisture Content (%)	5
Sample Number:	---	Final Moisture Content (%)	13
Depth (ft)	1 to 2	Initial Dry Density (pcf)	100.9
Specimen Diameter (in)	2.4	Final Dry Density (pcf)	126.1
Specimen Thickness (in)	1.0	Percent Collapse (%)	9.20

Prop. Moreno Valley Logistics Center
Moreno Valley, California
Project No. 14G160
PLATE C- 13

SoCalGeo
SOUTHERN CALIFORNIA GEOTECHNICAL
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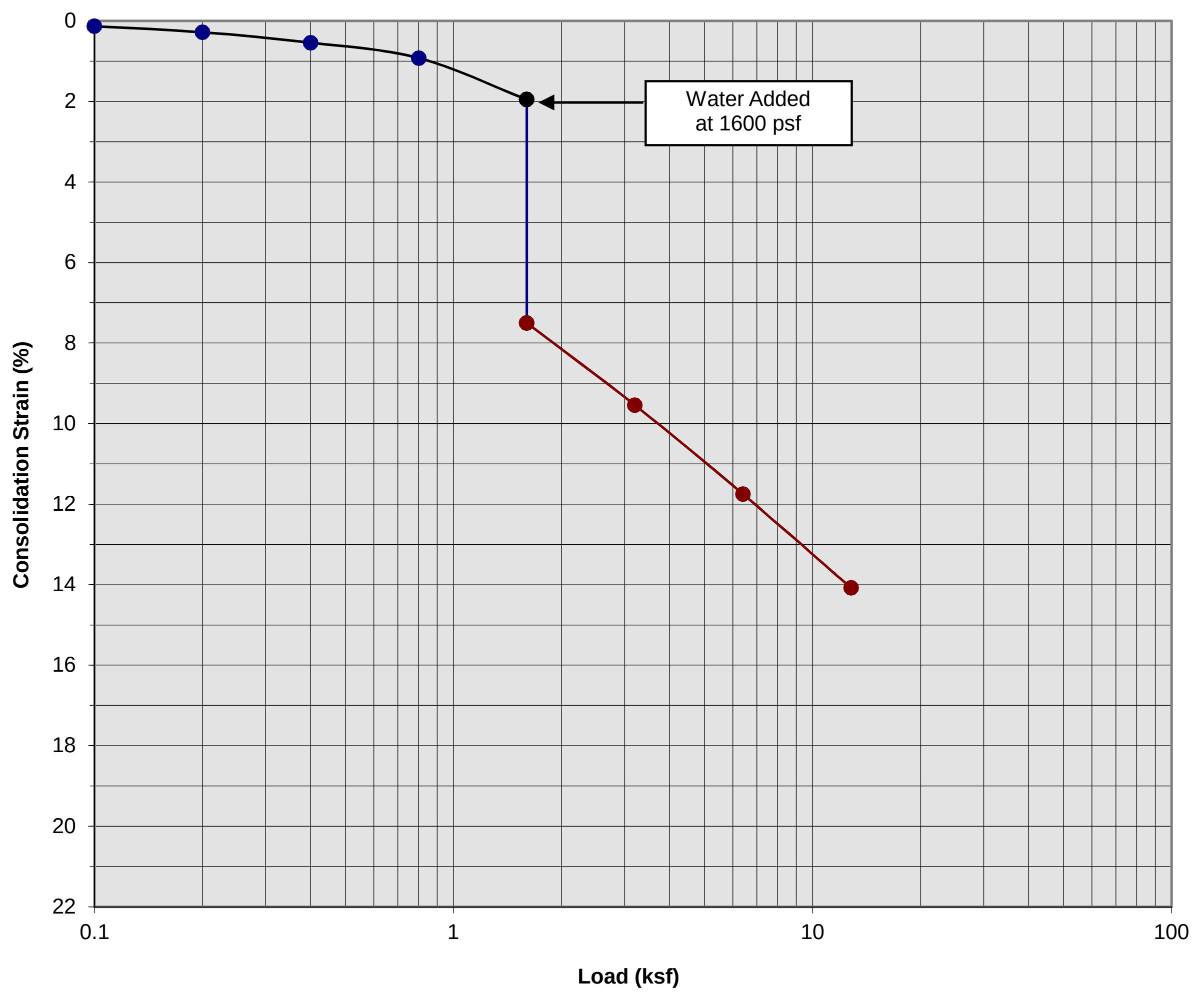
Consolidation/Collapse Test Results



Classification: Brown Silty fine to medium Sand, trace coarse Sand, trace Clay

Boring Number:	B-16	Initial Moisture Content (%)	5
Sample Number:	---	Final Moisture Content (%)	13
Depth (ft)	3 to 4	Initial Dry Density (pcf)	114.7
Specimen Diameter (in)	2.4	Final Dry Density (pcf)	132.5
Specimen Thickness (in)	1.0	Percent Collapse (%)	5.55

Prop. Moreno Valley Logistics Center
Moreno Valley, California
Project No. 14G160
PLATE C- 14

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SOUTHERN CALIFORNIA GEOTECHNICAL
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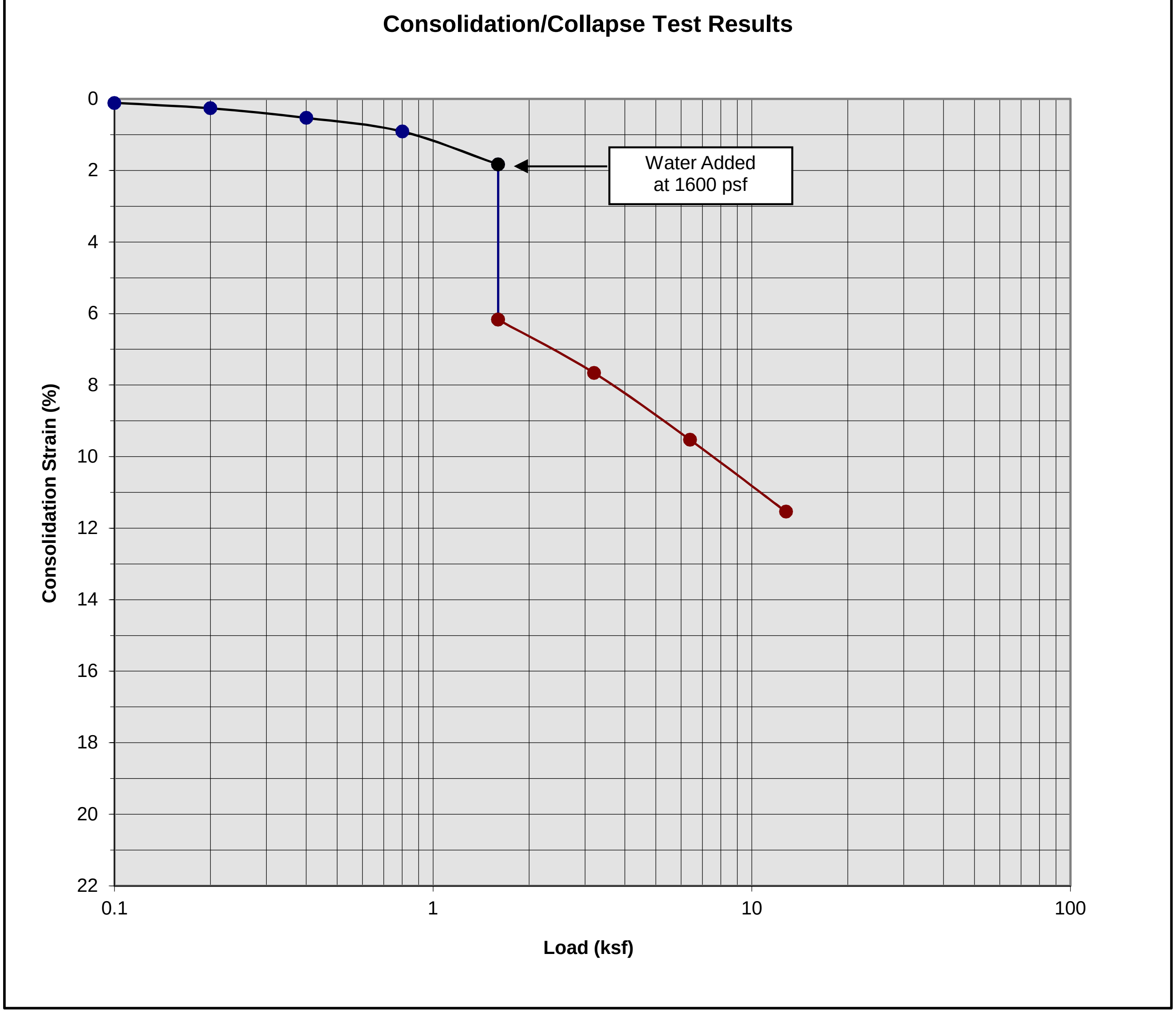

Classification: Brown fine to coarse Sand, some Silt

Boring Number:	B-16	Initial Moisture Content (%)	3
Sample Number:	---	Final Moisture Content (%)	14
Depth (ft)	5 to 6	Initial Dry Density (pcf)	109.1
Specimen Diameter (in)	2.4	Final Dry Density (pcf)	123.1
Specimen Thickness (in)	1.0	Percent Collapse (%)	4.34

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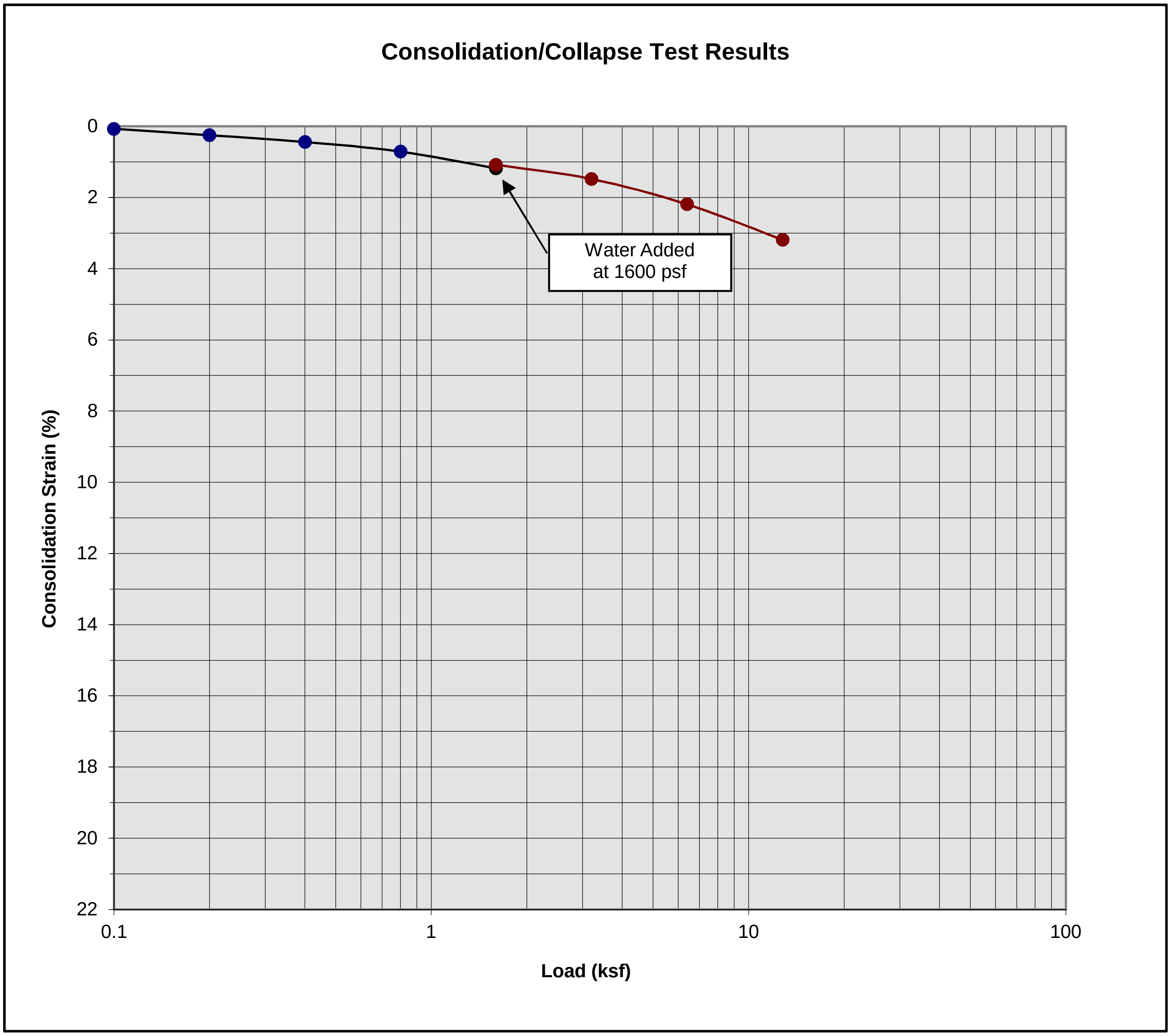

Classification: Dark Brown Silty Clay, trace fine Sand

Boring Number:	B-16	Initial Moisture Content (%)	12
Sample Number:	---	Final Moisture Content (%)	16
Depth (ft)	7 to 8	Initial Dry Density (pcf)	121.7
Specimen Diameter (in)	2.4	Final Dry Density (pcf)	125.4
Specimen Thickness (in)	1.0	Percent Collapse (%)	-0.10

Prop. Moreno Valley Logistics Center
Moreno Valley, California
Project No. 14G160
PLATE C- 16

SoCalGeo SOUTHERN CALIFORNIA GEOTECHNICAL
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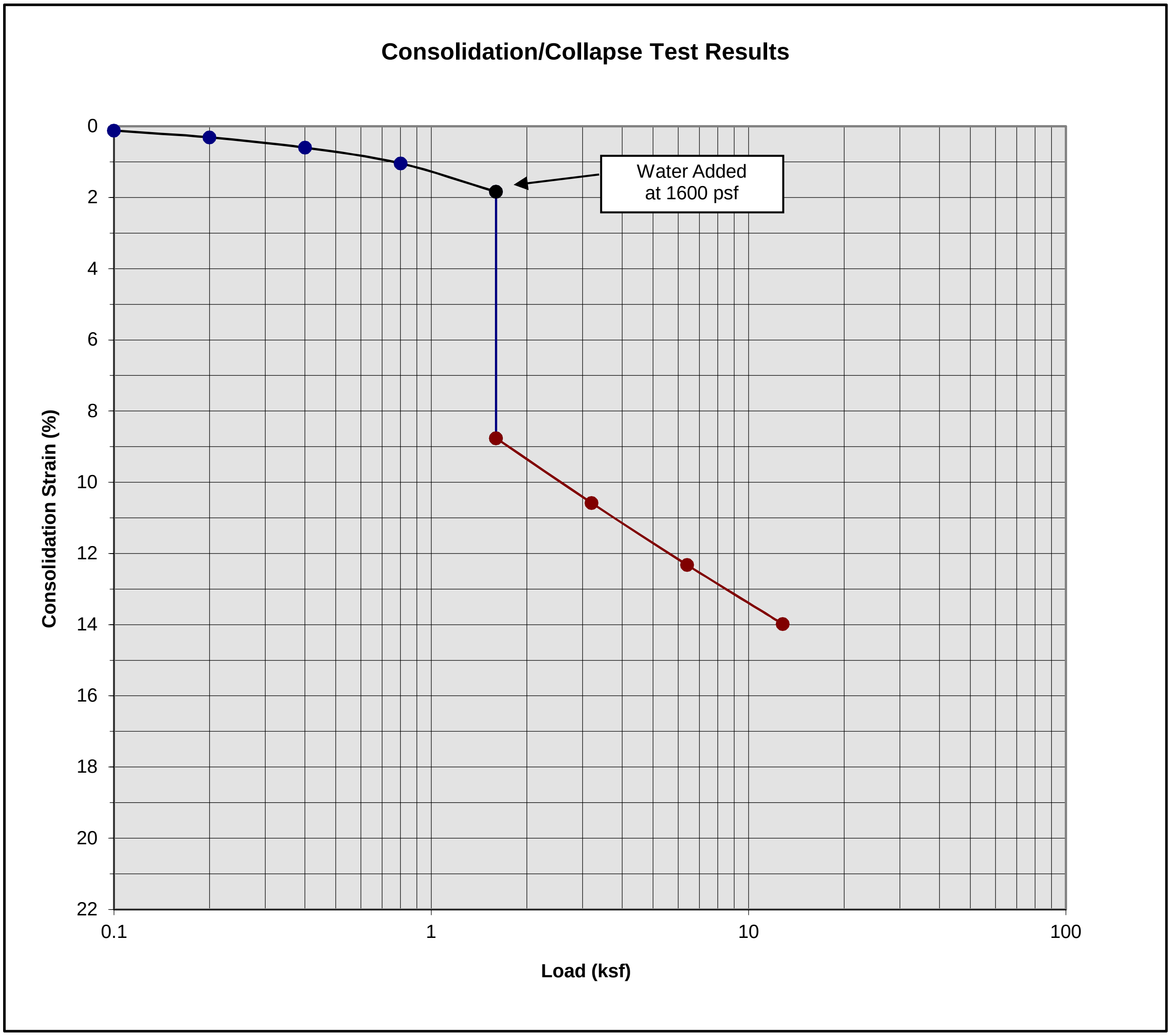

Classification: Gray Brown Silty fine Sand to fine Sandy Silt, trace coarse Sand

Boring Number:	B-19	Initial Moisture Content (%)	2
Sample Number:	---	Final Moisture Content (%)	14
Depth (ft)	1 to 2	Initial Dry Density (pcf)	105.0
Specimen Diameter (in)	2.4	Final Dry Density (pcf)	122.4
Specimen Thickness (in)	1.0	Percent Collapse (%)	6.92

Prop. Moreno Valley Logistics Center
Moreno Valley, California
Project No. 14G160
PLATE C- 17

SoCalGeo SOUTHERN CALIFORNIA GEOTECHNICAL
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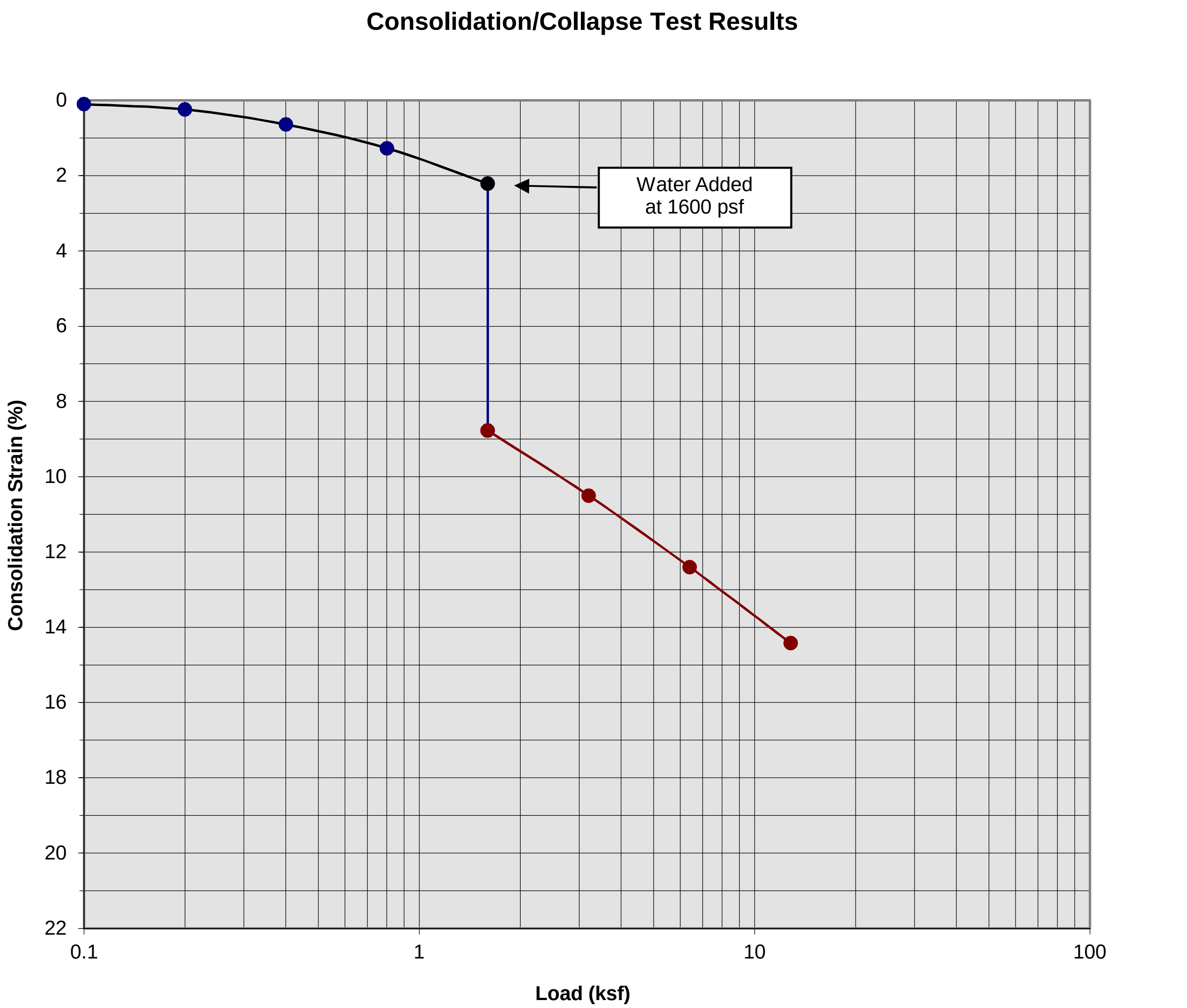

Classification: Brown Silty fine to medium Sand

Boring Number:	B-19	Initial Moisture Content (%)	2
Sample Number:	---	Final Moisture Content (%)	11
Depth (ft)	3 to 4	Initial Dry Density (pcf)	107.5
Specimen Diameter (in)	2.4	Final Dry Density (pcf)	125.8
Specimen Thickness (in)	1.0	Percent Collapse (%)	6.56

Prop. Moreno Valley Logistics Center
Moreno Valley, California
Project No. 14G160

PLATE C- 18

SoCalGeo SOUTHERN CALIFORNIA GEOTECHNICAL
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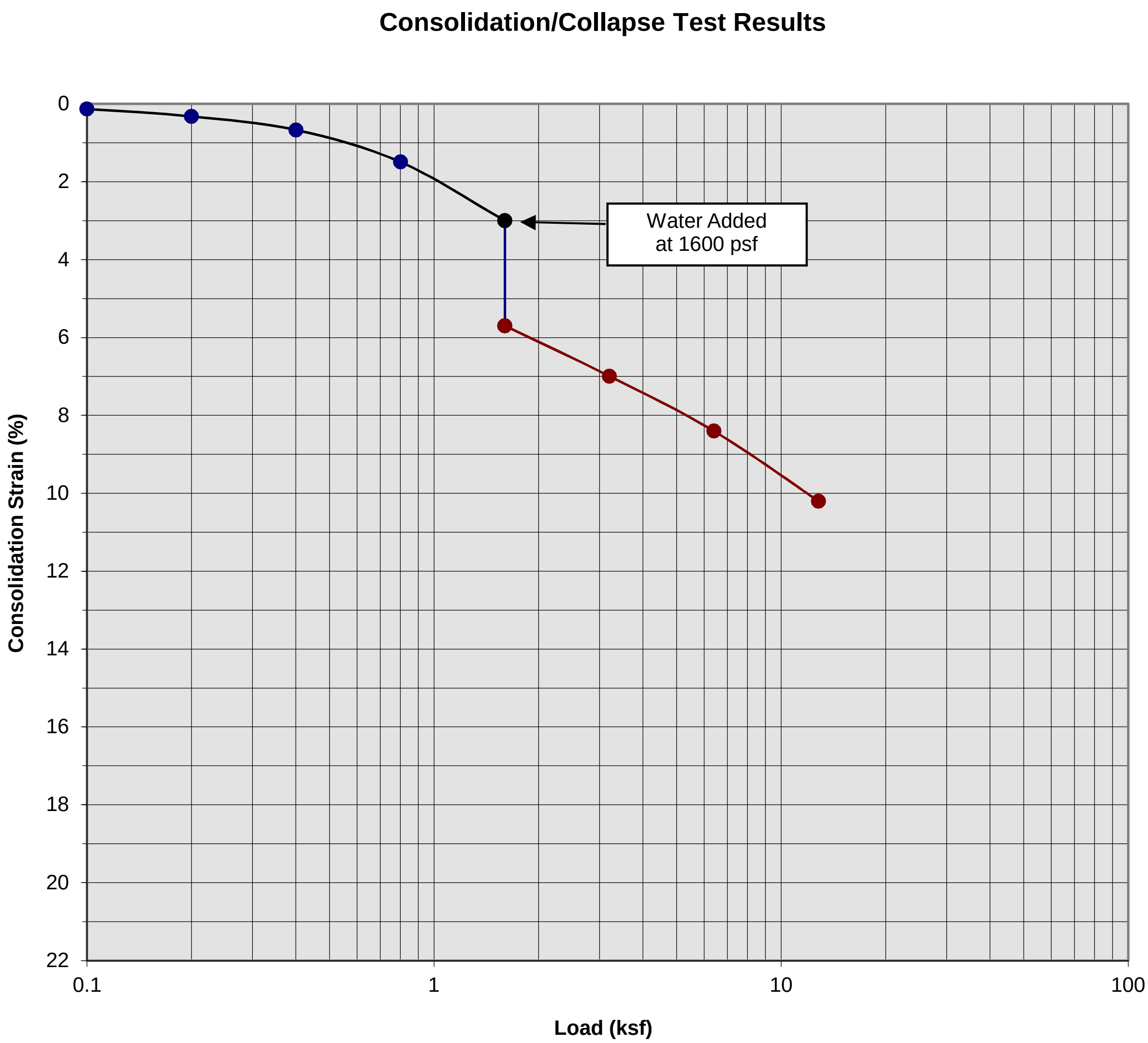
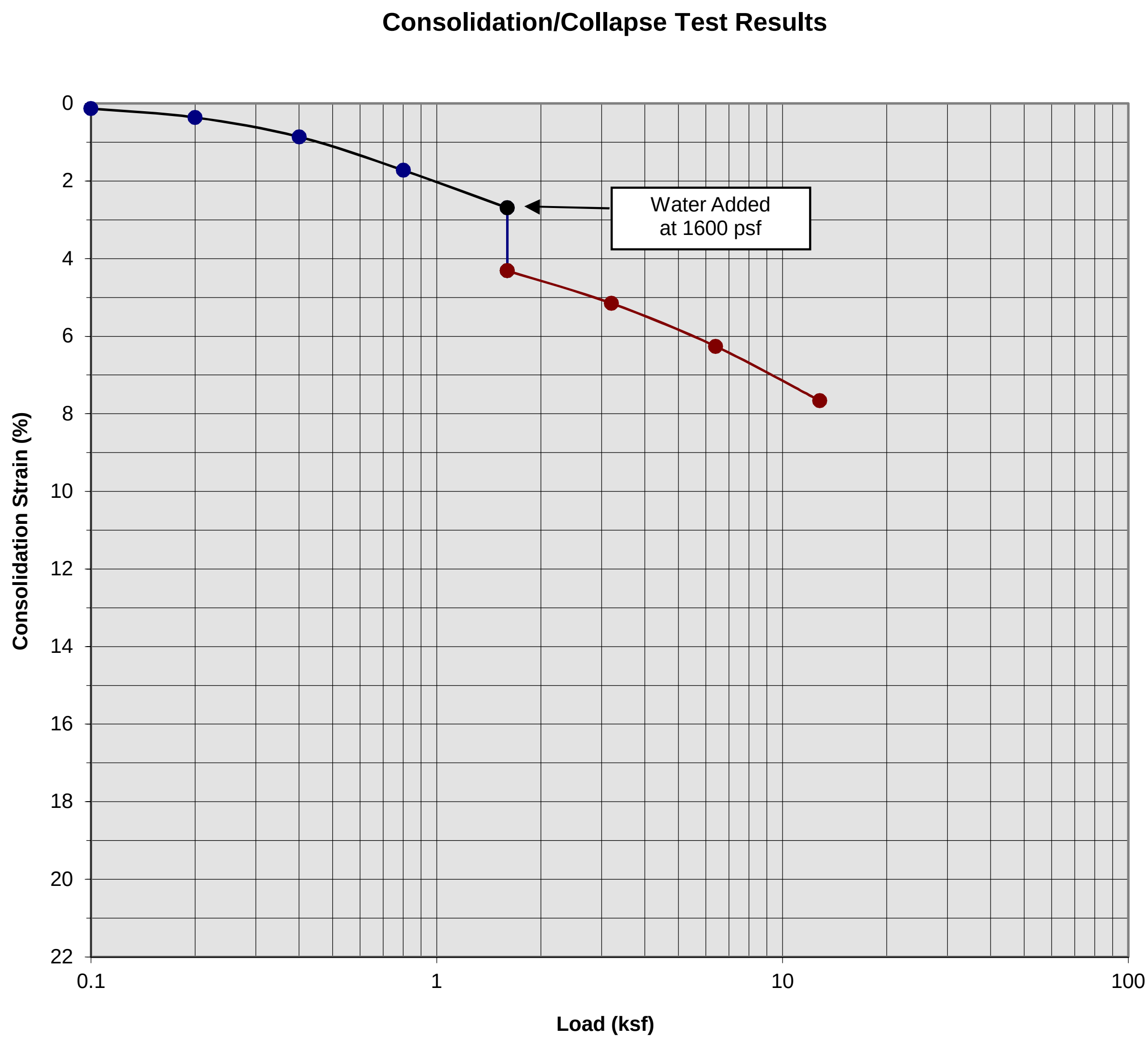


Classification: Gray Brown Silty fine Sand, trace Clay, little medium to coarse Sand

Boring Number:	B-19	Initial Moisture Content (%)	4
Sample Number:	---	Final Moisture Content (%)	14
Depth (ft)	5 to 6	Initial Dry Density (pcf)	117.7
Specimen Diameter (in)	2.4	Final Dry Density (pcf)	130.7
Specimen Thickness (in)	1.0	Percent Collapse (%)	2.70

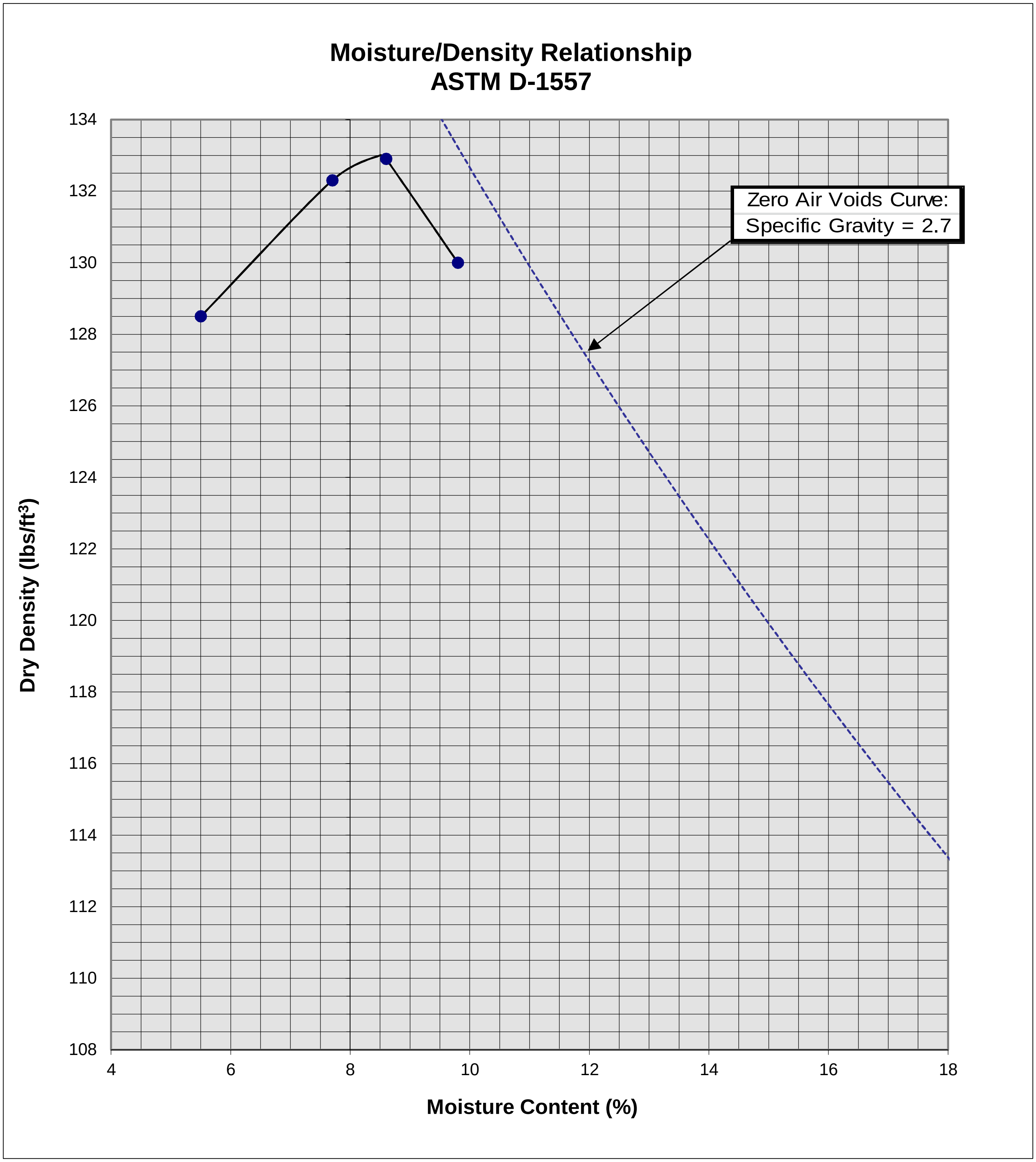
Prop. Moreno Valley Logistics Center
Moreno Valley, California
Project No. 14G160
PLATE C- 19

SoCalGeo SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation



Classification: Gray Brown Clayey fine Sand, trace Silt, trace to little medium Sand

Boring Number:	B-19	Initial Moisture Content (%)	6
Sample Number:	---	Final Moisture Content (%)	14
Depth (ft)	7 to 8	Initial Dry Density (pcf)	121.2
Specimen Diameter (in)	2.4	Final Dry Density (pcf)	131.6
Specimen Thickness (in)	1.0	Percent Collapse (%)	1.62



Soil ID Number	B-8 @ 0 to 5'
Optimum Moisture (%)	8.5
Maximum Dry Density (pcf)	133
Soil Classification	Brown Silty fine Sand, little Clay

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Moreno Valley, California
Project No. 14G160
PLATE C-21

SOUTHERN CALIFORNIA GEOTECHNICAL
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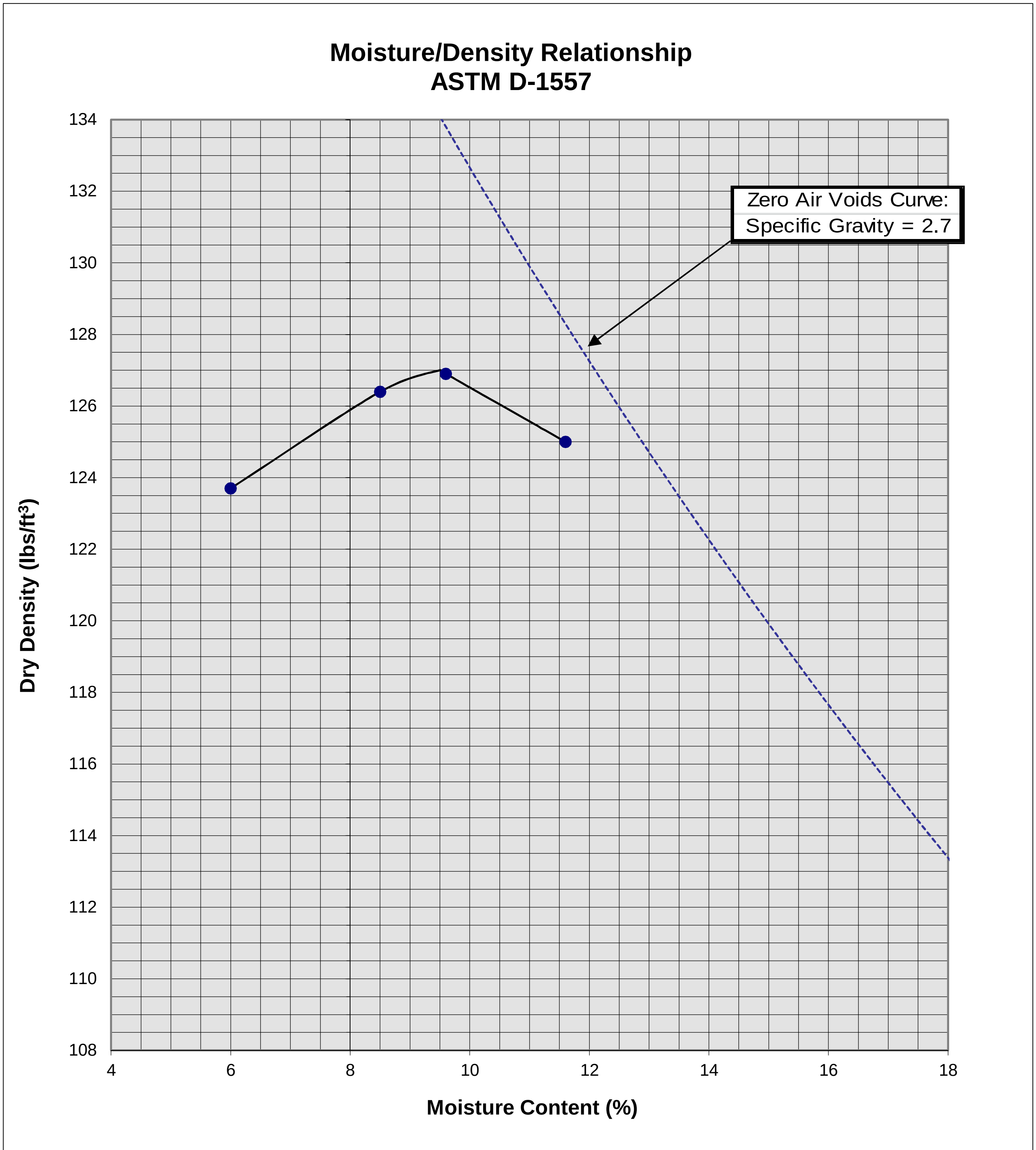
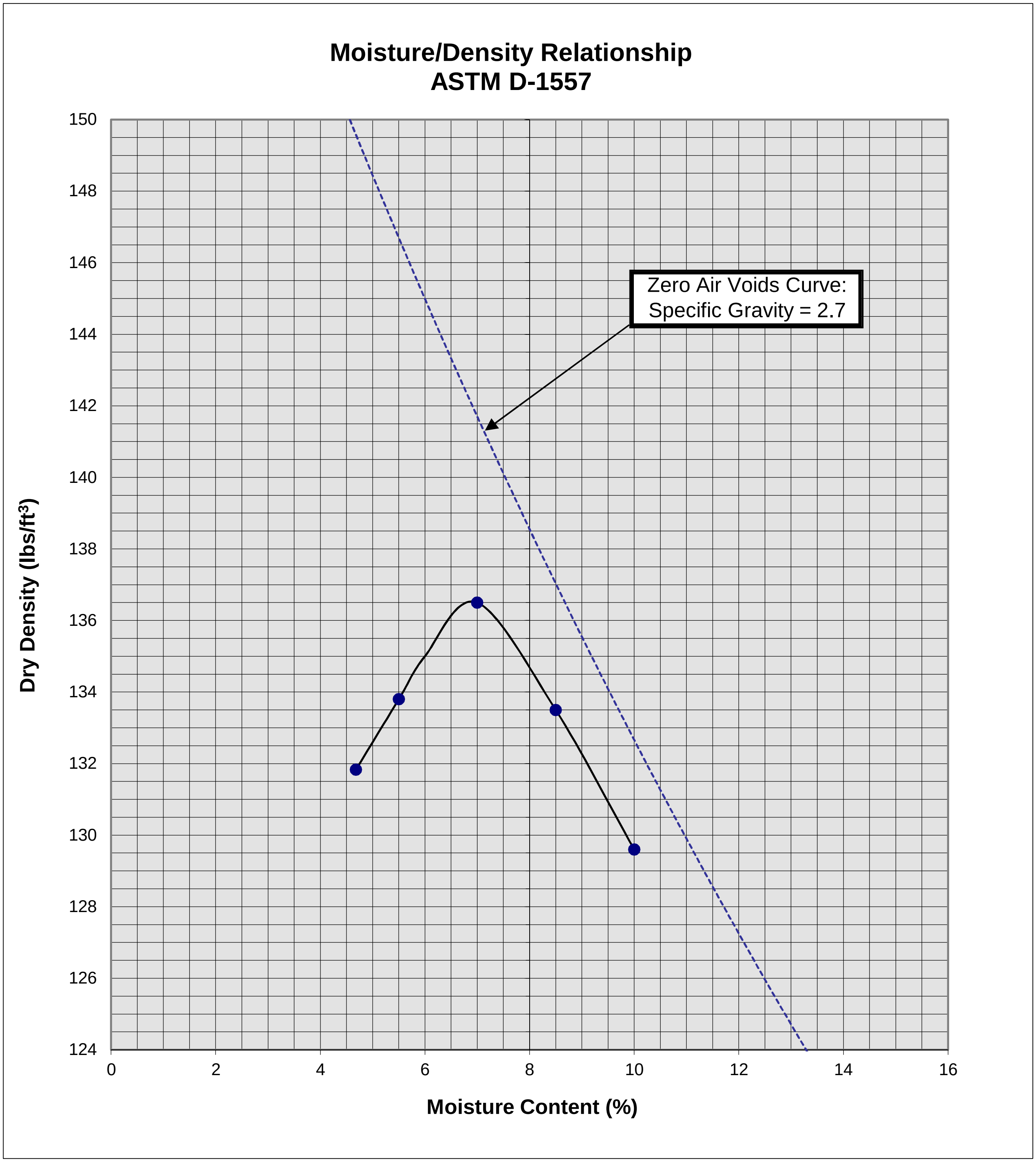


Soil ID Number	B-13 @ 0 to 3'
Optimum Moisture (%)	9.5
Maximum Dry Density (pcf)	127
Soil Classification	Dark Brown Silty fine Sand

Prop. Moreno Valley Logistics Center
Moreno Valley, California
Project No. 14G160
PLATE C-22

SoCalGeo SOUTHERN CALIFORNIA GEOTECHNICAL A California Corporation



Soil ID Number	B-21 @ 0 to 3'
Optimum Moisture (%)	7
Maximum Dry Density (pcf)	136.5
Soil Classification	Brown Silty fine to medium Sand, trace coarse Sand

Prop. Moreno Valley Logistics Center
Moreno Valley, California
Project No. 14G160
PLATE C-23

SOUTHERN CALIFORNIA GEOTECHNICAL
A California Corporation

APPENDIX D

GRADING GUIDE SPECIFICATIONS

These grading guide specifications are intended to provide typical procedures for grading operations. They are intended to supplement the recommendations contained in the geotechnical investigation report for this project. Should the recommendations in the geotechnical investigation report conflict with the grading guide specifications, the more site specific recommendations in the geotechnical investigation report will govern.

General

- The Earthwork Contractor is responsible for the satisfactory completion of all earthwork in accordance with the plans and geotechnical reports, and in accordance with city, county, and applicable building codes.

- The Geotechnical Engineer is the representative of the Owner/Builder for the purpose of implementing the report recommendations and guidelines. These duties are not intended to relieve the Earthwork Contractor of any responsibility to perform in a workman-like manner, nor is the Geotechnical Engineer to direct the grading equipment or personnel employed by the Contractor.

- The Earthwork Contractor is required to notify the Geotechnical Engineer of the anticipated work and schedule so that testing and inspections can be provided. If necessary, work may be stopped and redone if personnel have not been scheduled in advance.

- The Earthwork Contractor is required to have suitable and sufficient equipment on the job-site to process, moisture condition, mix and compact the amount of fill being placed to the approved compaction. In addition, suitable support equipment should be available to conform with recommendations and guidelines in this report.

- Canyon cleanouts, overexcavation areas, processed ground to receive fill, key excavations, subdrains and benches should be observed by the Geotechnical Engineer prior to placement of any fill. It is the Earthwork Contractor's responsibility to notify the Geotechnical Engineer of areas that are ready for inspection.

- Excavation, filling, and subgrade preparation should be performed in a manner and sequence that will provide drainage at all times and proper control of erosion. Precipitation, springs, and seepage water encountered shall be pumped or drained to provide a suitable working surface. The Geotechnical Engineer must be informed of springs or water seepage encountered during grading or foundation construction for possible revision to the recommended construction procedures and/or installation of subdrains.

Site Preparation

- The Earthwork Contractor is responsible for all clearing, grubbing, stripping and site preparation for the project in accordance with the recommendations of the Geotechnical Engineer.

- If any materials or areas are encountered by the Earthwork Contractor which are suspected of having toxic or environmentally sensitive contamination, the Geotechnical Engineer and Owner/Builder should be notified immediately.

- Major vegetation should be stripped and disposed of off-site. This includes trees, brush, heavy grasses and any materials considered unsuitable by the Geotechnical Engineer.

- Underground structures such as basements, cesspools or septic disposal systems, mining shafts, tunnels, wells and pipelines should be removed under the inspection of the Geotechnical Engineer and recommendations provided by the Geotechnical Engineer and/or city, county or state agencies. If such structures are known or found, the Geotechnical Engineer should be notified as soon as possible so that recommendations can be formulated.

- Any topsoil, slopewash, colluvium, alluvium and rock materials which are considered unsuitable by the Geotechnical Engineer should be removed prior to fill placement.

- Remaining voids created during site clearing caused by removal of trees, foundations basements, irrigation facilities, etc., should be excavated and filled with compacted fill.

- Subsequent to clearing and removals, areas to receive fill should be scarified to a depth of 10 to 12 inches, moisture conditioned and compacted

- The moisture condition of the processed ground should be at or slightly above the optimum moisture content as determined by the Geotechnical Engineer. Depending upon field conditions, this may require air drying or watering together with mixing and/or discing.

<u>Compacted Fills</u>

- Soil materials imported to or excavated on the property may be utilized in the fill, provided each material has been determined to be suitable in the opinion of the Geotechnical Engineer. Unless otherwise approved by the Geotechnical Engineer, all fill materials shall be free of deleterious, organic, or frozen matter, shall contain no chemicals that may result in the material being classified as “contaminated,” and shall be very low to non-expansive with a maximum expansion index (EI) of 50. The top 12 inches of the compacted fill should have a maximum particle size of 3 inches, and all underlying compacted fill material a maximum 6-inch particle size, except as noted below.

- All soils should be evaluated and tested by the Geotechnical Engineer. Materials with high expansion potential, low strength, poor gradation or containing organic materials may require removal from the site or selective placement and/or mixing to the satisfaction of the Geotechnical Engineer.

- Rock fragments or rocks less than 6 inches in their largest dimensions, or as otherwise determined by the Geotechnical Engineer, may be used in compacted fill, provided the distribution and placement is satisfactory in the opinion of the Geotechnical Engineer.

- Rock fragments or rocks greater than 12 inches should be taken off-site or placed in accordance with recommendations and in areas designated as suitable by the Geotechnical Engineer. These materials should be placed in accordance with Plate D-8 of these Grading Guide Specifications and in accordance with the following recommendations:

    - Rocks 12 inches or more in diameter should be placed in rows at least 15 feet apart, 15 feet from the edge of the fill, and 10 feet or more below subgrade. Spaces should be left between each rock fragment to provide for placement and compaction of soil around the fragments.

    - Fill materials consisting of soil meeting the minimum moisture content requirements and free of oversize material should be placed between and over the rows of rock or concrete. Ample water and compactive effort should be applied to the fill materials as they are placed in order that all of the voids between each of the fragments are filled and compacted to the specified density.

- Subsequent rows of rocks should be placed such that they are not directly above a row placed in the previous lift of fill. A minimum 5-foot offset between rows is recommended.

- To facilitate future trenching, oversized material should not be placed within the range of foundation excavations, future utilities or other underground construction unless specifically approved by the soil engineer and the developer/owner representative.

- Fill materials approved by the Geotechnical Engineer should be placed in areas previously prepared to receive fill and in evenly placed, near horizontal layers at about 6 to 8 inches in loose thickness, or as otherwise determined by the Geotechnical Engineer for the project.

- Each layer should be moisture conditioned to optimum moisture content, or slightly above, as directed by the Geotechnical Engineer. After proper mixing and/or drying, to evenly distribute the moisture, the layers should be compacted to at least 90 percent of the maximum dry density in compliance with ASTM D-1557-78 unless otherwise indicated.

- Density and moisture content testing should be performed by the Geotechnical Engineer at random intervals and locations as determined by the Geotechnical Engineer. These tests are intended as an aid to the Earthwork Contractor, so he can evaluate his workmanship, equipment effectiveness and site conditions. The Earthwork Contractor is responsible for compaction as required by the Geotechnical Report(s) and governmental agencies.

- Fill areas unused for a period of time may require moisture conditioning, processing and recompaction prior to the start of additional filling. The Earthwork Contractor should notify the Geotechnical Engineer of his intent so that an evaluation can be made.

- Fill placed on ground sloping at a 5-to-1 inclination (horizontal-to-vertical) or steeper should be benched into bedrock or other suitable materials, as directed by the Geotechnical Engineer. Typical details of benching are illustrated on Plates D-2, D-4, and D-5.

- Cut/fill transition lots should have the cut portion overexcavated to a depth of at least 3 feet and rebuilt with fill (see Plate D-1), as determined by the Geotechnical Engineer.

- All cut lots should be inspected by the Geotechnical Engineer for fracturing and other bedrock conditions. If necessary, the pads should be overexcavated to a depth of 3 feet and rebuilt with a uniform, more cohesive soil type to impede moisture penetration.

- Cut portions of pad areas above buttresses or stabilizations should be overexcavated to a depth of 3 feet and rebuilt with uniform, more cohesive compacted fill to impede moisture penetration.

- Non-structural fill adjacent to structural fill should typically be placed in unison to provide lateral support. Backfill along walls must be placed and compacted with care to ensure that excessive unbalanced lateral pressures do not develop. The type of fill material placed adjacent to below grade walls must be properly tested and approved by the Geotechnical Engineer with consideration of the lateral earth pressure used in the design.

Foundations

- The foundation influence zone is defined as extending one foot horizontally from the outside edge of a footing, and proceeding downward at a ½ horizontal to 1 vertical (0.5:1) inclination.
- Where overexcavation beneath a footing subgrade is necessary, it should be conducted so as to encompass the entire foundation influence zone, as described above.
- Compacted fill adjacent to exterior footings should extend at least 12 inches above foundation bearing grade. Compacted fill within the interior of structures should extend to the floor subgrade elevation.

Fill Slopes

- The placement and compaction of fill described above applies to all fill slopes. Slope compaction should be accomplished by overfilling the slope, adequately compacting the fill in even layers, including the overfilled zone and cutting the slope back to expose the compacted core
- Slope compaction may also be achieved by backrolling the slope adequately every 2 to 4 vertical feet during the filling process as well as requiring the earth moving and compaction equipment to work close to the top of the slope. Upon completion of slope construction, the slope face should be compacted with a sheepsfoot connected to a sideboom and then grid rolled. This method of slope compaction should only be used if approved by the Geotechnical Engineer.
- Sandy soils lacking in adequate cohesion may be unstable for a finished slope condition and therefore should not be placed within 15 horizontal feet of the slope face.
- All fill slopes should be keyed into bedrock or other suitable material. Fill keys should be at least 15 feet wide and inclined at 2 percent into the slope. For slopes higher than 30 feet, the fill key width should be equal to one-half the height of the slope (see Plate D-5).
- All fill keys should be cleared of loose slough material prior to geotechnical inspection and should be approved by the Geotechnical Engineer and governmental agencies prior to filling.
- The cut portion of fill over cut slopes should be made first and inspected by the Geotechnical Engineer for possible stabilization requirements. The fill portion should be adequately keyed through all surficial soils and into bedrock or suitable material. Soils should be removed from the transition zone between the cut and fill portions (see Plate D-2).

Cut Slopes

- All cut slopes should be inspected by the Geotechnical Engineer to determine the need for stabilization. The Earthwork Contractor should notify the Geotechnical Engineer when slope cutting is in progress at intervals of 10 vertical feet. Failure to notify may result in a delay in recommendations.
- Cut slopes exposing loose, cohesionless sands should be reported to the Geotechnical Engineer for possible stabilization recommendations.
- All stabilization excavations should be cleared of loose slough material prior to geotechnical inspection. Stakes should be provided by the Civil Engineer to verify the location and dimensions of the key. A typical stabilization fill detail is shown on Plate D-5.

- Stabilization key excavations should be provided with subdrains. Typical subdrain details are shown on Plates D-6.

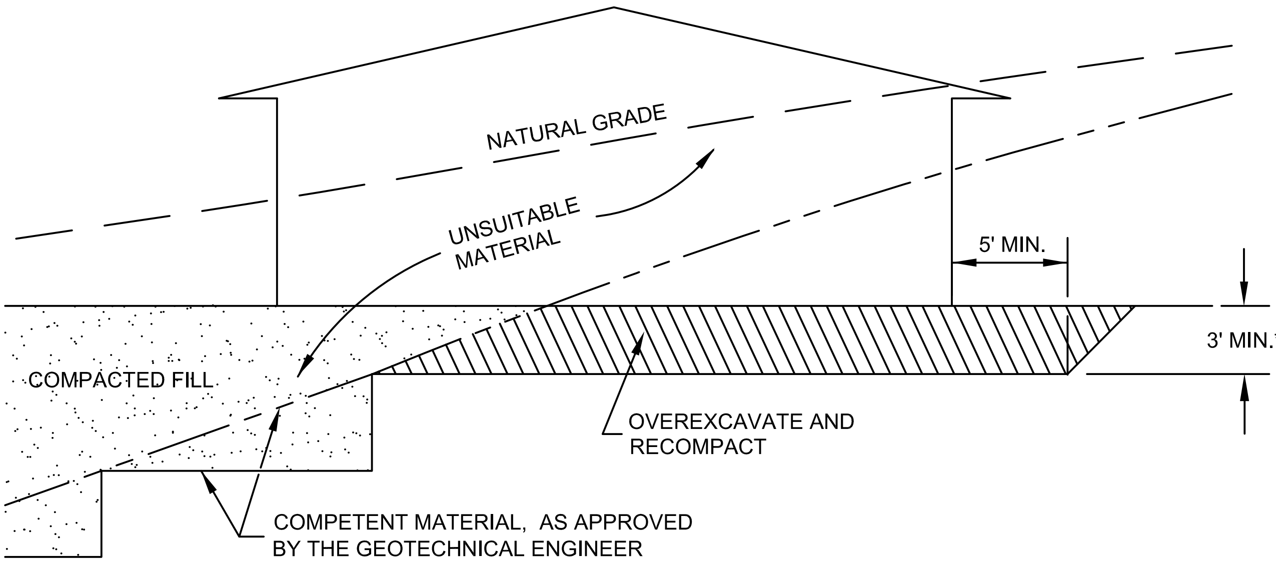
Subdrains

- Subdrains may be required in canyons and swales where fill placement is proposed. Typical subdrain details for canyons are shown on Plate D-3. Subdrains should be installed after approval of removals and before filling, as determined by the Soils Engineer.

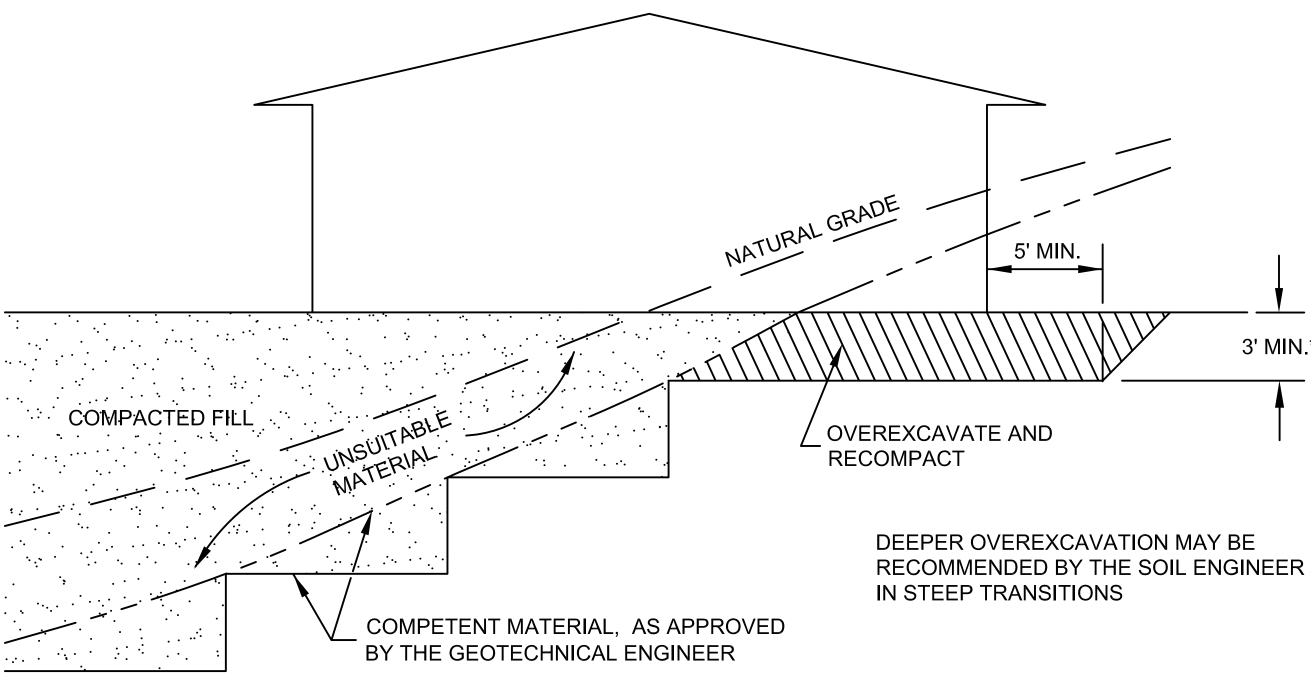
- Plastic pipe may be used for subdrains provided it is Schedule 40 or SDR 35 or equivalent. Pipe should be protected against breakage, typically by placement in a square-cut (backhoe) trench or as recommended by the manufacturer.

- Filter material for subdrains should conform to CALTRANS Specification 68-1.025 or as approved by the Geotechnical Engineer for the specific site conditions. Clean ¾-inch crushed rock may be used provided it is wrapped in an acceptable filter cloth and approved by the Geotechnical Engineer. Pipe diameters should be 6 inches for runs up to 500 feet and 8 inches for the downstream continuations of longer runs. Four-inch diameter pipe may be used in buttress and stabilization fills.

CUT LOT



CUT/FILL LOT (TRANSITION)



*SEE TEXT OF REPORT FOR SPECIFIC RECOMMENDATION.
ACTUAL DEPTH OF OVEREXCAVATION MAY BE GREATER.

TRANSITION LOT DETAIL

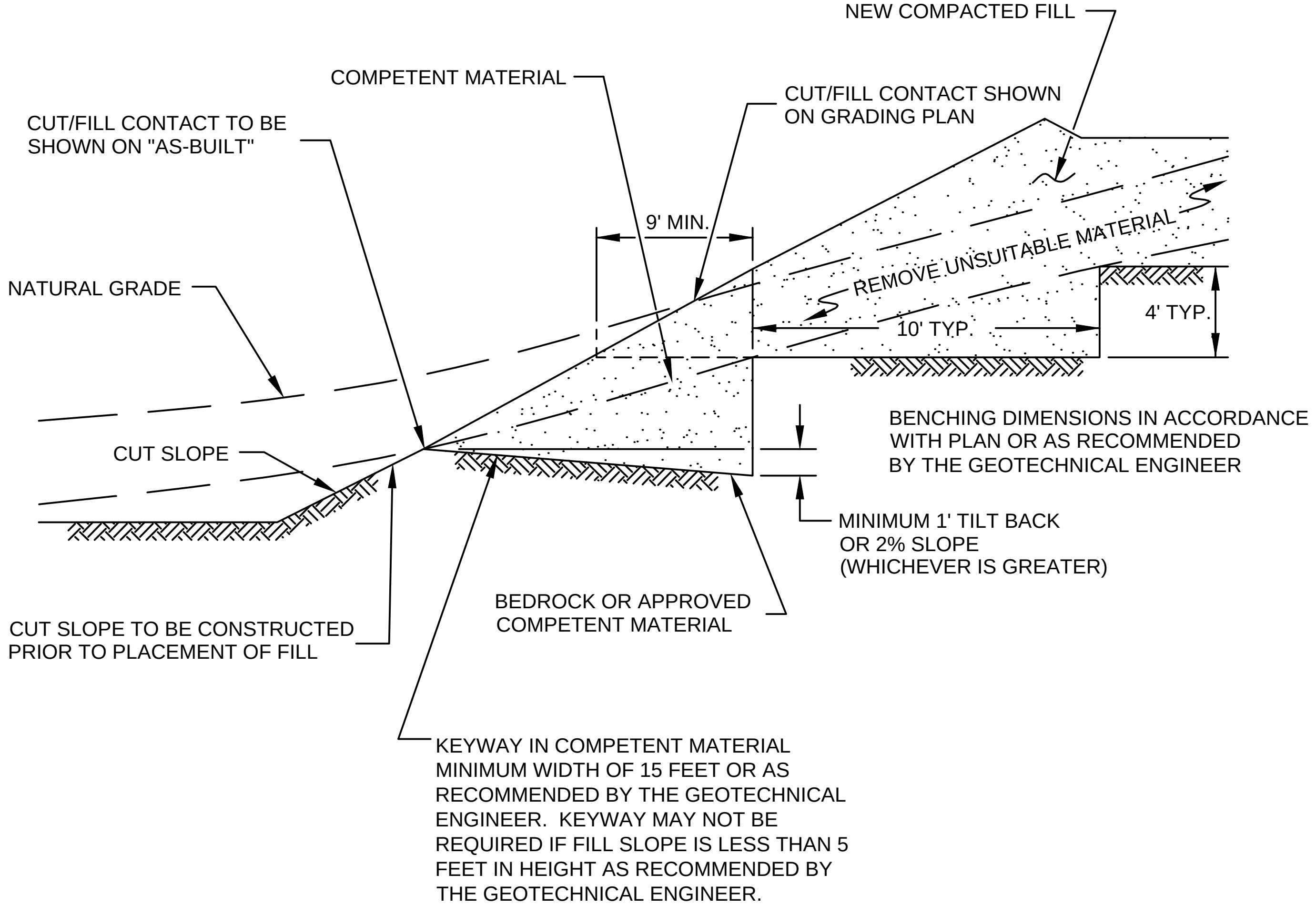
GRADING GUIDE SPECIFICATIONS

NOT TO SCALE

DRAWN: JAS
CHKD: GKM

PLATE D-1

SoCalGeo
SOUTHERN CALIFORNIA GEOTECHNICAL



FILL ABOVE CUT SLOPE DETAIL

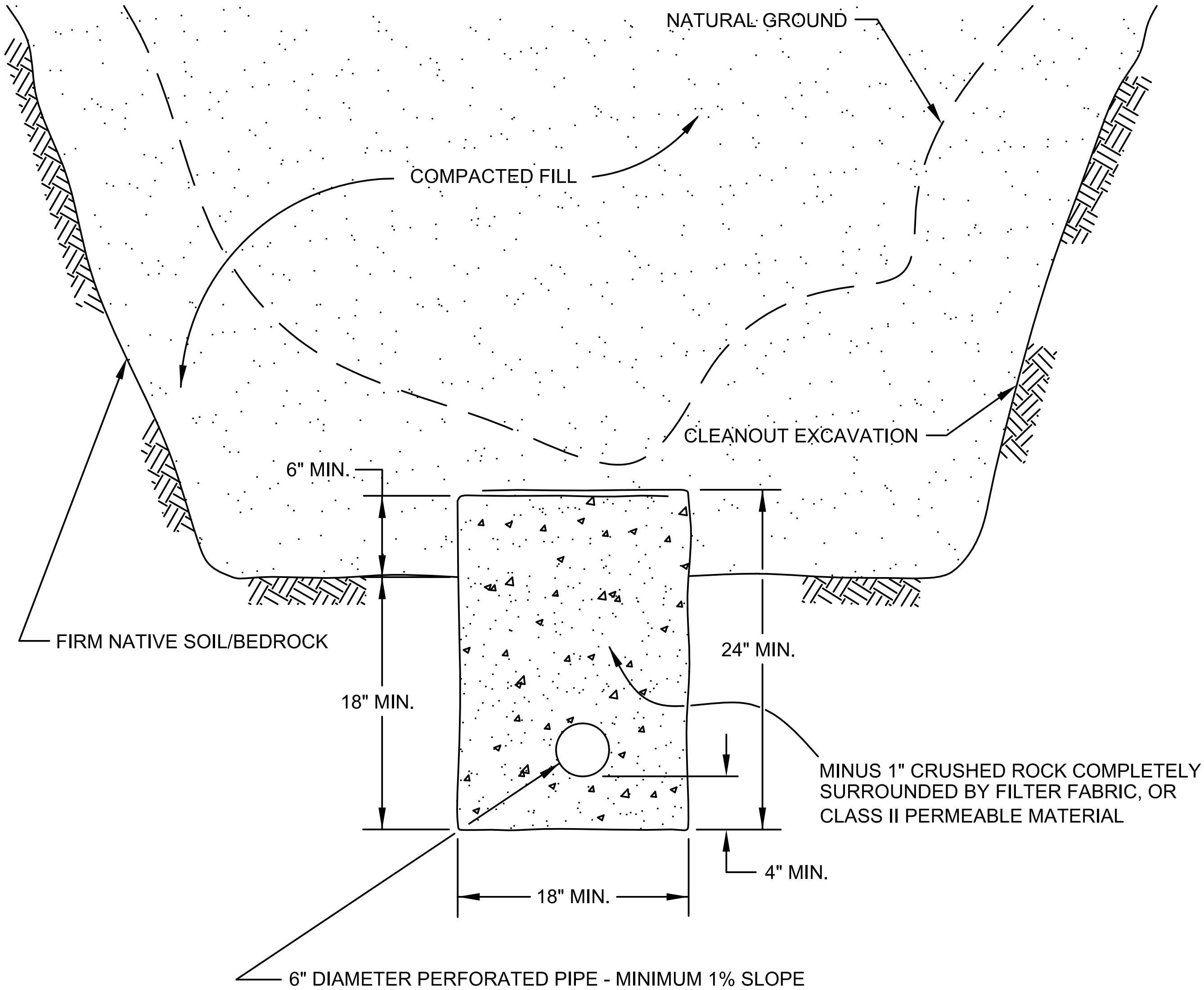
GRADING GUIDE SPECIFICATIONS

NOT TO SCALE

DRAWN: JAS
CHKD: GKM

PLATE D-2

SOUTHERN CALIFORNIA GEOTECHNICAL



PIPE MATERIAL	DEPTH OF FILL OVER SUBDRAIN
ADS (CORRUGATED POLETHYLENE)	8
TRANSITE UNDERDRAIN	20
PVC OR ABS: SDR 35	35
SDR 21	100

SCHEMATIC ONLY
NOT TO SCALE

CANYON SUBDRAIN DETAIL

GRADING GUIDE SPECIFICATIONS

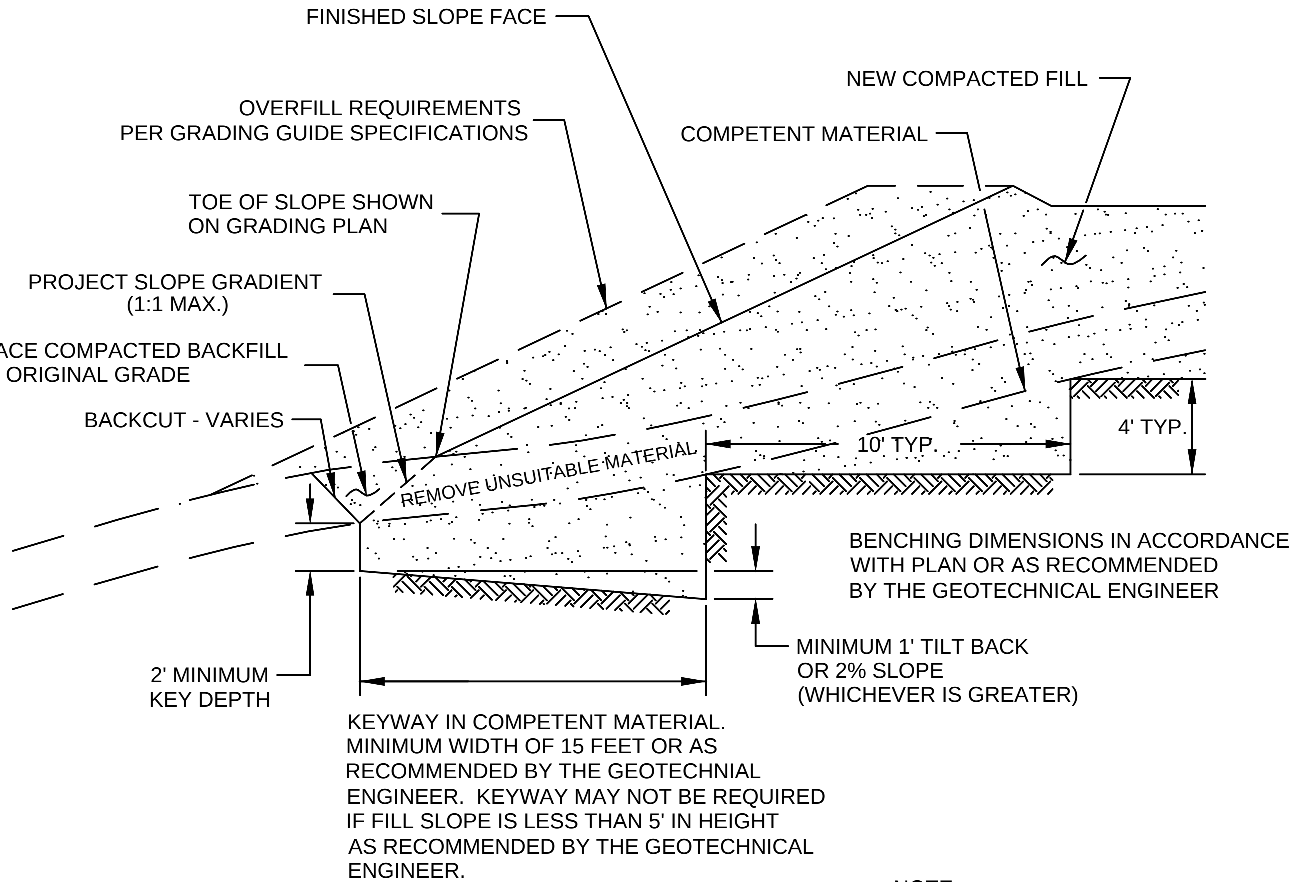
NOT TO SCALE

DRAWN: JAS
CHKD: GKM

PLATE D-3

SoCalGeo

SOUTHERN CALIFORNIA GEOTECHNICAL



NOTE:
BENCHING SHALL BE REQUIRED WHEN NATURAL SLOPES ARE EQUAL TO OR STEEPER THAN 5:1 OR WHEN RECOMMENDED BY THE GEOTECHNICAL ENGINEER.

FILL ABOVE NATURAL SLOPE DETAIL

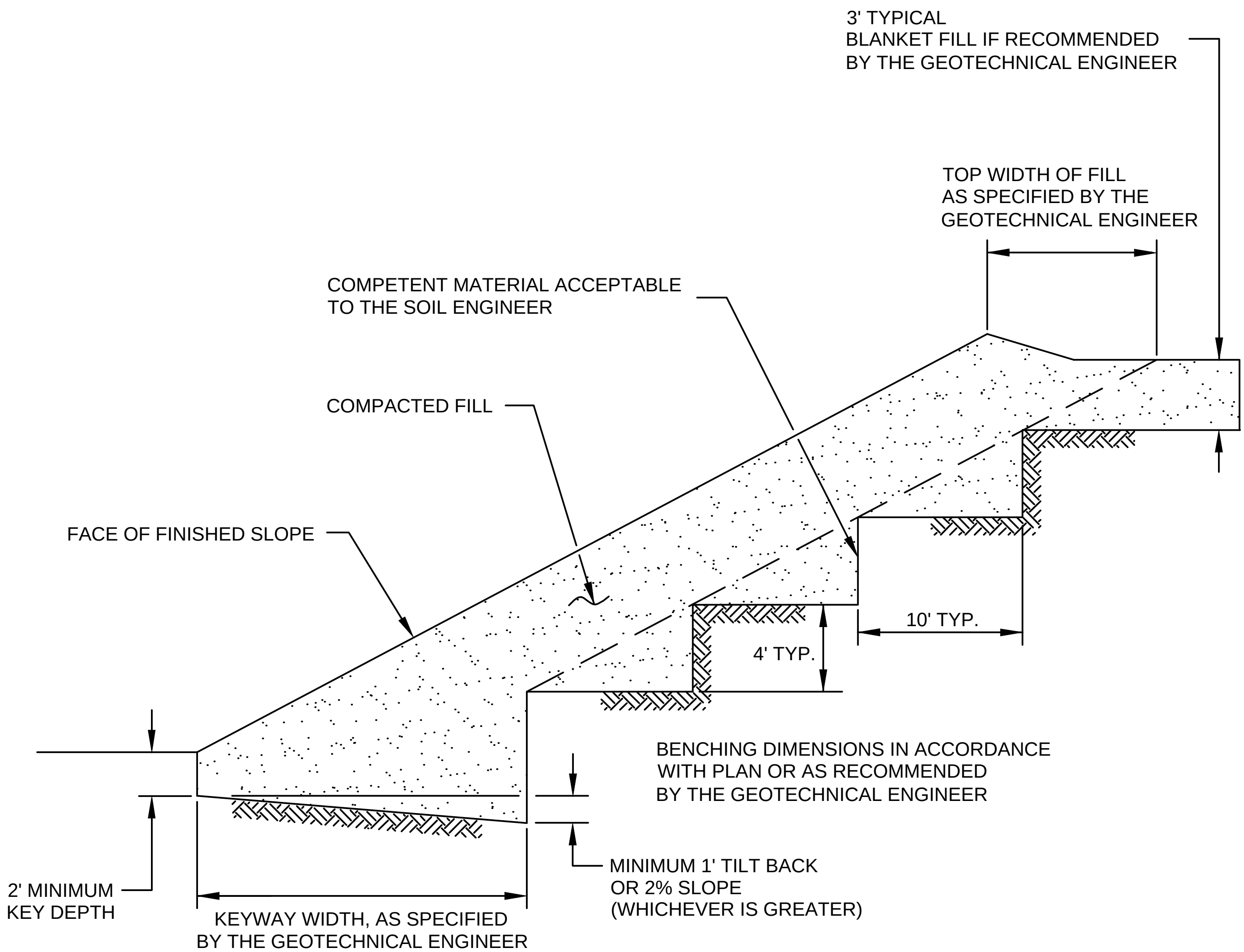
GRADING GUIDE SPECIFICATIONS

NOT TO SCALE

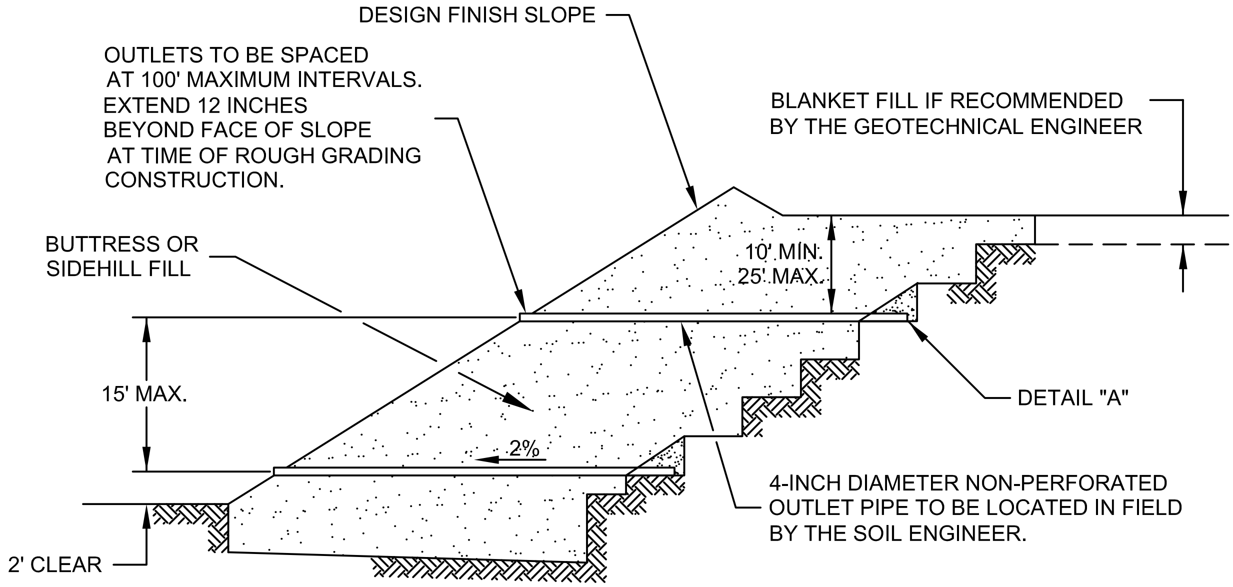
DRAWN: JAS
CHKD: GKM

PLATE D-4

SOUTHERN CALIFORNIA GEOTECHNICAL



STABILIZATION FILL DETAIL	
GRADING GUIDE SPECIFICATIONS	
NOT TO SCALE	SOUTHERN CALIFORNIA GEOTECHNICAL
DRAWN: JAS CHKD: GKM	
PLATE D-5	



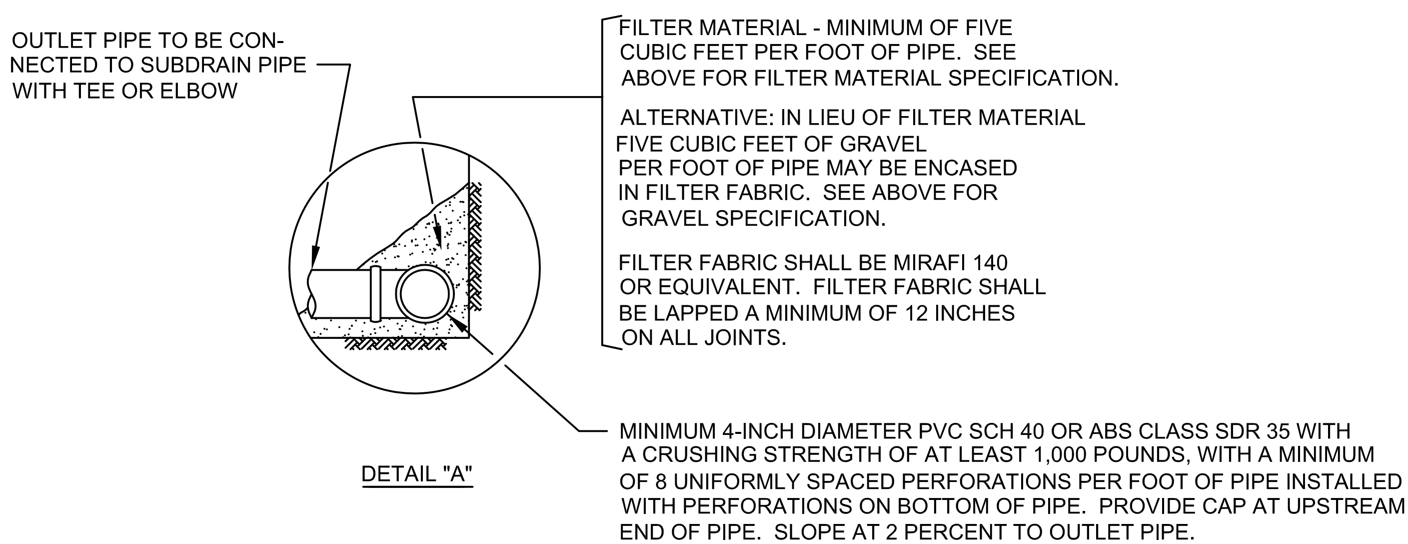
"FILTER MATERIAL" TO MEET FOLLOWING SPECIFICATION OR APPROVED EQUIVALENT: (CONFORMS TO EMA STD. PLAN 323)

SIEVE SIZE	PERCENTAGE PASSING
1"	100
3/4"	90-100
3/8"	40-100
NO. 4	25-40
NO. 8	18-33
NO. 30	5-15
NO. 50	0-7
NO. 200	0-3

"GRAVEL" TO MEET FOLLOWING SPECIFICATION OR APPROVED EQUIVALENT:

SIEVE SIZE	MAXIMUM PERCENTAGE PASSING
1 1/2"	100
NO. 4	50
NO. 200	8

SAND EQUIVALENT = MINIMUM OF 50



DETAIL "A"

NOTES:
1. TRENCH FOR OUTLET PIPES TO BE BACKFILLED WITH ON-SITE SOIL.

SLOPE FILL SUBDRAINS

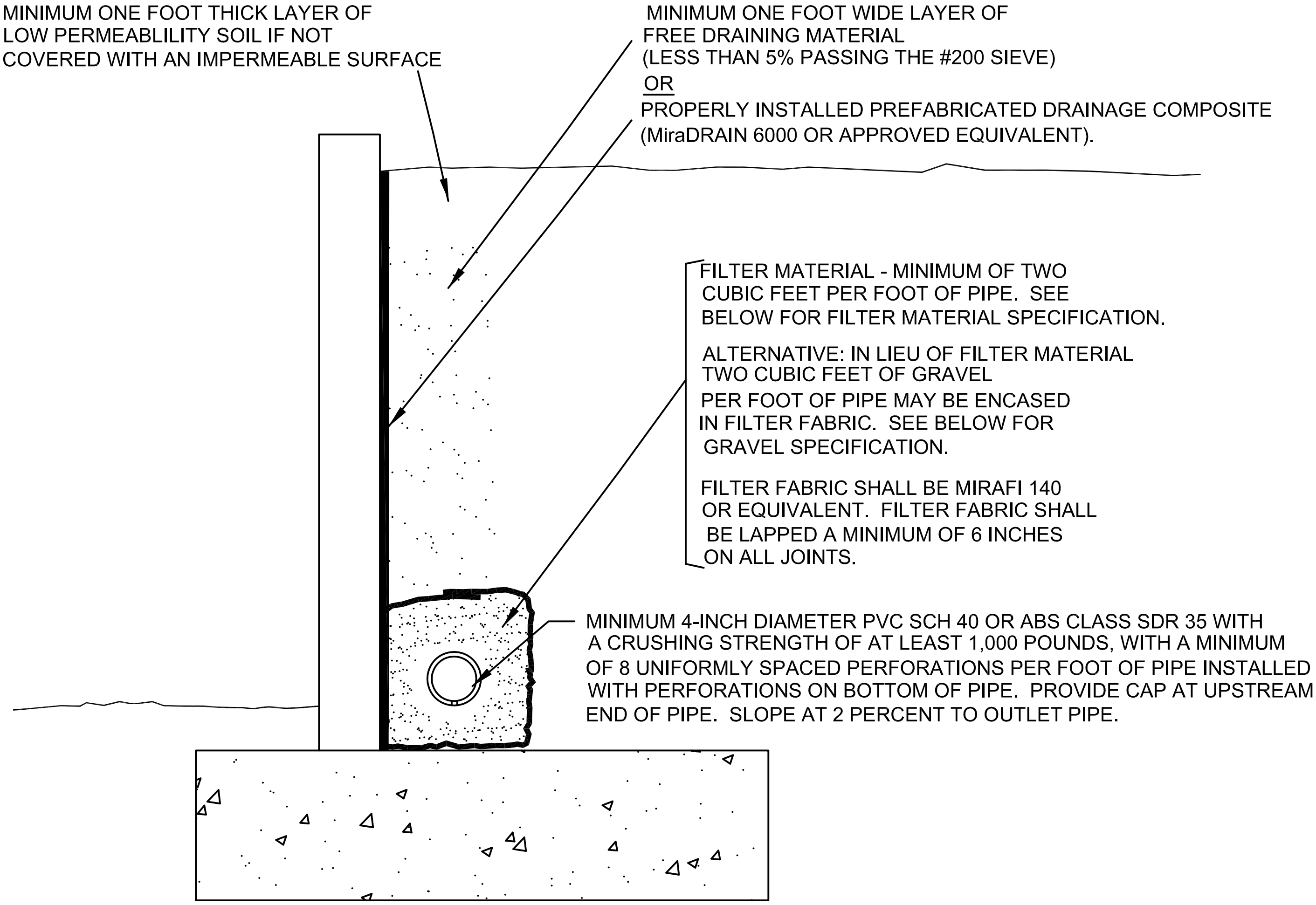
GRADING GUIDE SPECIFICATIONS

NOT TO SCALE

DRAWN: JAS
CHKD: GKM

PLATE D-6

SoCalGeo
SOUTHERN CALIFORNIA GEOTECHNICAL



"FILTER MATERIAL" TO MEET FOLLOWING SPECIFICATION
OR APPROVED EQUIVALENT: (CONFORMS TO EMA STD. PLAN 323)

SIEVE SIZE	PERCENTAGE PASSING
1"	100
3/4"	90-100
3/8"	40-100
NO. 4	25-40
NO. 8	18-33
NO. 30	5-15
NO. 50	0-7
NO. 200	0-3

"GRAVEL" TO MEET FOLLOWING SPECIFICATION OR
APPROVED EQUIVALENT:

SIEVE SIZE	MAXIMUM PERCENTAGE PASSING
1 1/2"	100
NO. 4	50
NO. 200	8

SAND EQUIVALENT = MINIMUM OF 50

RETAINING WALL BACKDRAINS

GRADING GUIDE SPECIFICATIONS

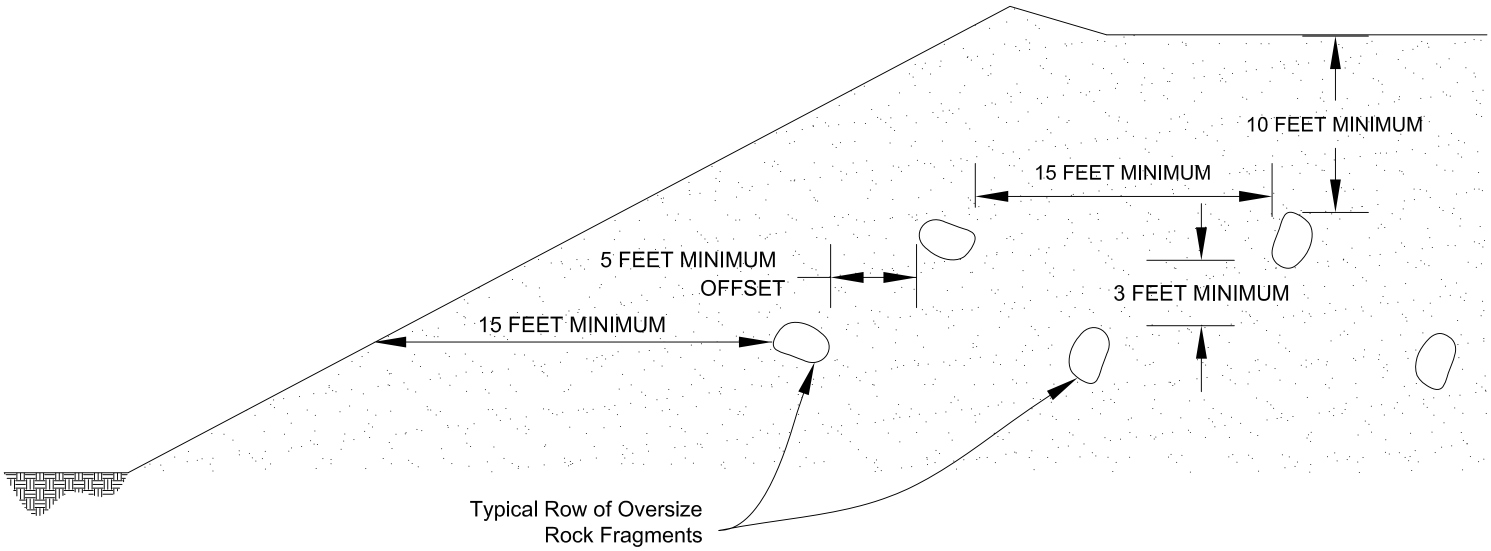
NOT TO SCALE

DRAWN: JAS
CHKD: GKM

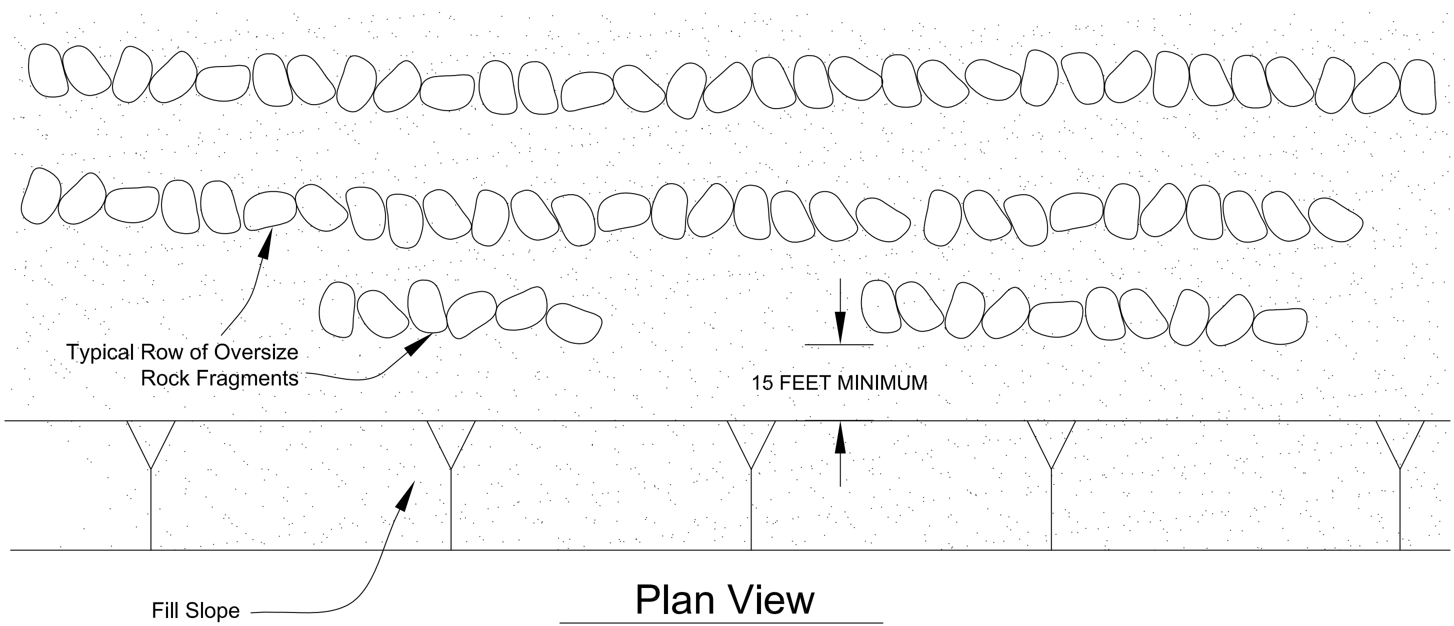
PLATE D-7

SoCalGeo

SOUTHERN CALIFORNIA GEOTECHNICAL



Section View



Plan View

PLACEMENT OF OVERSIZED MATERIAL

GRADING GUIDE SPECIFICATIONS

NOT TO SCALE

DRAWN: PM
CHKD: GKM

PLATE D-8

SOUTHERN CALIFORNIA GEOTECHNICAL

APPENDIX E

USGS Design Maps Summary Report

User–Specified Input

Report Title Proposed Moreno Valley Logistics Center
Fri March 20, 2015 17:24:25 UTC

Building Code Reference Document ASCE 7-10 Standard
(which utilizes USGS hazard data available in 2008)

Site Coordinates 33.87639°N, 117.23689°W

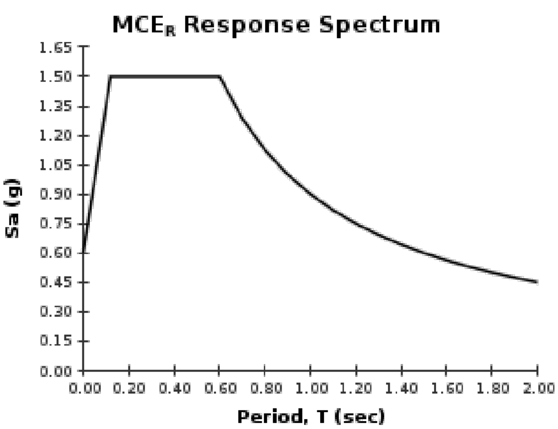
Site Soil Classification Site Class D – "Stiff Soil"

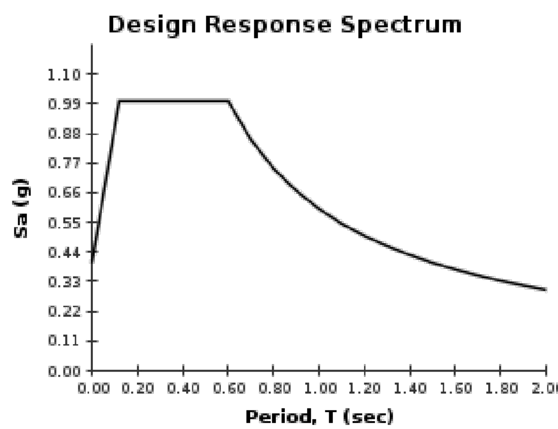
Risk Category I/II/III

USGS–Provided Output

S_S =	1.500 g	S_{MS} =	1.500 g	S_{DS} =	1.000 g
S_1 =	0.600 g	S_{M1} =	0.900 g	S_{D1} =	0.600 g

For information on how the SS and S1 values above have been calculated from probabilistic (risk-targeted) and deterministic ground motions in the direction of maximum horizontal response, please return to the application and select the "2009 NEHRP" building code reference document.





SOURCE: U.S. GEOLOGICAL SURVEY (USGS)
<http://geohazards.usgs.gov/designmaps/us/application.php>

SEISMIC DESIGN PARAMETERS

PROPOSED MORENO VALLEY LOGISTICS CENTER

MORENO VALLEY, CALIFORNIA

DRAWN: PM
CHKD: JAS

SCG PROJECT
14G160-3

PLATE E-1

SoCalGeo
SOUTHERN CALIFORNIA GEOTECHNICAL